BASELINE RISK ASSESSMENT AND GROUNDWATER QUALITY REPORT

3126 Brinkerhoff Road, Kansas City, Kansas

Prepared by:

Cyrus B. Parks, Environmental Engineer
Union Carbide Corporation
Health, Safety, and Environmental Technology
P.O. Box 8361
South Charleston, WV 25303

Reviewed by:

Dr. K.C. Lee, Senior Research Scientist (above address)

D.H. Ott Operations and Environmental Affairs Manager (below address)

Submitted by:

UNISON Transformer Services, Inc. 7401 Carmel Executive Park, Suite 206 Charlotte, NC 28226

September, 1994







September 14,1994

SEP 15 1994

Mr. Dennis Grams
Regional Administrator
United States EPA, Region VII
726 Minnesota Ave.

Kansas City, Kansas 66101

Re: Unison Transformer Services, Inc.: EP ID No. KSD980688733 Submittal of Risk Assessment; TSCA Closure Site at Brinkerhoff Rd., Kansas City, KS

Dear Mr. Grams:

Unison Transformer Services, Inc.(Unison) has been conducting site closure activities under an EPA approved TSCA closure plan at the subject facility since early 1992. Significant remediation work was necessary, and has been completed, primarily to meet PCB clean-up criteria. Unison has kept EPA informed of progress through communication with Mr. Robert Jackson of the Toxic Substances Control Section.

As a brief overview of the remediation status at the Brinkerhoff site, most of the activity has been associated with the removal of the concrete floor inside the building and the underlying soil to a nominal depth of two feet. Overall, nearly 2700 tons of PCB contaminated material has been removed and landfilled in an EPA approved TSCA landfill. The entire site encompasses 32000 square feet, including the outside parking lot areas. Sample results obtained in accordance with the approved EPA sample protocol show that, with the exception of an isolated area (about 500 sq ft) inside the building, the site meets the clean-up criteria for PCB. During closure activities, this small isolated area was discovered to be contaminated with both trichloroethylene (TCE) and PCB. About 500 tons of the removed soil underlying this area was treated to remove TCE prior to landfilling. This activity has been completed, but not all sample results under the established protocol for TCE and PCB meet the clean-up criteria at the bottom of the excavation some twenty feet below the original floor. Safety concerns and structural risks to the building limited the practical extent of the excavation.

Unison has developed and conducted the enclosed formal site study and risk assessment to address the possible impact of the small amount of residual contamination that remains in the isolated area about twenty feet below the original grade level of the site. As detailed in the report, site remediation as currently completed eliminates any significant health and environmental risk related to the presence of residual TCE or PCB. Therefore, the attached "Baseline Risk Assessment and Groundwater Quality Report" is being submitted by Unison to EPA for consideration in gaining final Agency approval for closure and release from the requirements for financial assurance.

Your prompt attention to this submittal is appreciated. Unison would be willing to meet with appropriate EPA representatives to discuss any aspects of the risk assessment or the closure status in general. Please direct any questions or requests to me at 704-541-4430.

Sincerely,

R.A.Ream

Vice President of Operations Unison Transformer Services, Inc.

P.O.Box 471008

Charlotte, NC 28247

Copies: Robert Jackson, US EPA VII

M.S.Liggatt, Unison D.H.Ott, Unison

BASELINE RISK ASSESSMENT AND GROUNDWATER QUALITY REPORT

3126 Brinkerhoff Road, Kansas City, Kansas

Prepared by:

Cyrus B. Parks, Environmental Engineer
Union Carbide Corporation
Health, Safety, and Environmental Technology
P.O. Box 8361
South Charleston, WV 25303

Reviewed by:

Dr. K.C. Lee, Senior Research Scientist (above address)

D.H. Ott Operations and Environmental Affairs Manager (below address)

Submitted by:

UNISON Transformer Services, Inc. 7401 Carmel Executive Park, Suite 206 Charlotte, NC 28226

September, 1994

TABLE OF CONTENTS

Summary of Findings	
1.0 Introduction	1-1
2.0 Site Description	2-1
2.1 Setting	2-1
2.2 History of Operations	2-1
2.3 Past Remediation	2-2
2.4 Present Course of Action	2-4
3.0 Local Geology	3-1
3.1 Regional Physiography	3-1
3.2 Regional Soils	3-2
3.3 Regional Bedrock	3-3
3.4 Site and Near-Site Bedrock	3-5
3.5 Site and Near-Site Alluvial Sediments	3-6
4.0 Local Hydrology	4-1
4.1 Regional Hydrogeology	4-1
4.1.1 Regional Upland Soil Aquifers	4-1
4.1.2 Regional Bedrock Aquifers	4-1
4.1.3 Regional Alluvial Aquifers	4-2
4.1.4 Regional Alluvial Aquifer Characteristics	4-3
4.2 Site and Near-Site Alluvial Aquifer4-5	
5.0 Site Groundwater Investigation	5-1
5.1 Site Investigation	
5.1.1 Borings, Monitoring Well and Piezometer Installa	ation 5-2
5.1.2 Groundwater Level Measurements	5-4
5.1.3 Groundwater Sampling Procedures	5-4
5.2 Site/Near-Site Groundwater Gradient and Flow	5-5
5.3 Analytical Results	5-6
5.4 Groundwater Quality	5-7
6.0 Chemicals of Concern	6-1
6.1 PCB	6-1
6.2 TCE	6-2
7.0 Contaminant Transport Analysis	7-1
7.1 Contaminant Pathway Analysis	7-1
7.1.1 Inhalation of Vapors and Particulates	7-2
7.1.2 Soil Ingestion	7-2
7.1.3 Dermal Contact	7-3

7.1.4 Water Ingestion	7-3		
7.2 Groundwater Pathway Analysis			
7.3 Fate and Transport Analysis			
7.4 Model Results			
7.4.1 TCE Model Results	7-5		
7.4.2 PCB Model Results	7-6		
7.5 Modeling Conservatism	7-7		
7.6 Flood Analysis	7-8		
8.0 Current and Future Land Use	8-1		
9.0 Risk Characterization	9-1		
9.1 Selection of Maximum Allowable Exposure Concentrations	9-1		
9.2 Risk Characterization	9-2		
10.0 Conclusions	10-1		
References			
Tables			
Figures			
Appendices			
A. Letter from Bowden Contracting Company, Inc. (3/14/94)			
B. Monitoring Well Installation Diagrams and Drilling Logs			
C. Analytical Results (Pace Laboratories)			
D. Material Safety Data Sheets			
E. Fate and Transport Modeling Results			
F. Flood Scenario Calculations			
G. PCB Risk Calculations			

LIST OF TABLES

Table 2-1: Soil Sampling Results

Table 3-1: Regional Pennsylvanian Stratigraphic Column

Table 4-1: Regional Bedrock Aquifer Quality and Yields

Table 4-2: Generalized/Average Regional Aquifer Characteristics

Table 4-3: Generalized Site/Near-Site Alluvial Aquifer Characteristics

Table 4-4: Site Alluvial Aquifer Characteristics

Table 4-5: Characteristics of Floodplain Soils

Table 5-1: Well/Piezometer Data

Table 5-2: Groundwater Elevations

Table 5-3: Summary of Analytical Laboratory Results

Table 6-1: PCB Chemical/Physical Properties

Table 6-2: TCE Chemical/Physical Properties

Table 7-1: RISKPRO, AT123D Model Inputs

LIST OF FIGURES

- Figure 2-1: Fairfax District, Kansas City, Kansas
- Figure 2-2: UNISON Transformer Services, Inc., 3126 Brinkerhoff Road
- Figure 2-3: Missouri River, Groundwater Elevations
- Figure 3-1: Regional Physiographic Subprovinces and Glacial Limits
- Figure 3-2: Regional Soil Profile
- Figure 3-3: Alluvial Soil Profile
- Figure 3-4: Regional Geologic Map
- Figure 3-5: Bedrock Topography
- Figure 3-6: Near-Site Alluvial Cross Section
- Figure 5-1: Expected Fairfax District Flow Map
- Figure 5-2: Site Groundwater Elevations (5/19/94)
- Figure 5-3: TCE Distribution
- Figure 5-4: DCE/TCE Ratios
- Figure 6-1: TCE Dehalogenation
- Figure 7-1: Model Results, TCE Year 11 (1994)
- Figure 7-2: Model Results, TCE Year 11 (1994) Small Scale
- Figure 7-3: Model Results, TCE Year 26 (2009)
- Figure 7-4: Model Results, TCE Year 41 (2024)
- Figure 7-5: Model Results, PCB Year 11 (1994)
- Figure 7-6: Model Results, PCB Year 26 (2009)
- Figure 7-7: Model Results, PCB Year 71 (2054)

SUMMARY OF FINDINGS

UNISON Transformer Services, Inc., entered into a facility operating agreement with Envirosure, Inc., on July 28, 1988. UNISON operated this transformer decontamination and reclamation facility from 1988 to 1991 in the Fairfax Industrial District of Kansas City, Kansas. As part of closure, UNISON conducted remediation activities to excavate and landfill approximately 2200 tons of soil and asphalt which was contaminated with polychlorinated biphenyls (PCB). PCB contaminated metal from various equipment and ductwork were removed and landfilled. Contaminated soil from the northwest corner of the building was excavated to groundwater level (about 20 ft below ground surface) and removed. Trichloroethylene (TCE) and PCB contamination from this area of the facility is believed to have occurred from 1983-1986, before UNISON operated the facility and chemical storage and handling facilities were upgraded. A small amount of contaminated soil in the northwest corner could not be excavated safely due to site limitations. The groundwater quality at the site was investigated for chemical contamination after PCB and TCE contaminated soils were found to go to the water table. UNISON contracted with ERM - Rocky Mountain, Inc., to conduct the investigation, which consisted of:

- 1 exploratory boring to determine soil stratigraphy;
- 4 piezometers to give a preliminary indication of groundwater gradient; and
- 6 monitoring wells to determine chemical distribution.

The site is located in the Missouri River floodplain. The topography is dominated by the floodplains and bluffs of the Missouri River. The first bedrock is made mostly of shales and is about 70 ft thick at the site, rendering it relatively impermeable. No faults are found in the area. The soils at the site are alluvial in nature and can be broken down into three regions:

- Upper 40% of vertical profile silt, clay, and very fine sand;
- Middle 40% medium sand with interlayers of silt and coarse sand; and
- Lower 20% coarse sand and gravel.

Findings indicate that TCE has dispersed from the source to the property boundaries, primarily eastward, in the direction of groundwater flow. The measured groundwater gradient is 4.0 feet per mile (0.076 %). Within 120 ft of the source, TCE concentrations diminish to approximately 1% of the highest values. The observed detection of PCB in the groundwater is not conclusive, as the samples were not filtered prior to analysis, and the small concentrations (max. 0.023 mg/L) may be due to PCB sorbed to soil particles in the sample water. Decreased chemical concentrations in the deeper wells indicate little vertical migration of these chemicals. No free phase material was found.

The only credible exposure pathway to subsurface contamination was found to be water ingestion. Fate and transport modeling was used to assess the risk to human health from PCB and TCE in the groundwater. The conservative estimates of the aquifer properties used in the fate and transport modeling have a basis in the data collected and the groundwater models developed at or near the Fairfax District. Because groundwater ingestion was found to be the only credible exposure pathway, the Maximum Contaminant Level (MCL) for PCB (0.0005 mg/L) was used to present a conservative estimate of groundwater contamination. The MCL for TCE (0.005 mg/L) was used to define the TCE groundwater plume.

Even under the many conservative assumptions and scenarios for contaminant transport, groundwater contamination is predicted to not impact human health or the environment. The forward edge of the modeled TCE plume, defined by the 0.005 mg/L line, will not move beyond 150 m (500 ft) from the source before completely dissipating. Both modeling and field data indicate that PCB will not migrate beyond the immediate source area.

The entire Fairfax District is zoned as Heavy Industrial and is likely to remain so in the future. There are no residential areas within the Fairfax District, and no groundwater from the Fairfax District is used for municipal purposes. Despite highly conservative assumptions and scenarios for exposure, no chemical contamination is predicted to reach or otherwise impact a receptor now and into the future. Site remediation as currently completed has substantially eliminated health and environmental risk related to the presence of TCE or PCB.

1.0 INTRODUCTION

This report documents the site investigation and risk assessment of the 3126 Brinkerhoff Road site in Kansas City, Kansas, for the purpose of determining remedial strategy. Objectives include:

- determining levels and distributions of chemicals in the subsurface,
- assessing risk from such chemicals, and
- presenting basis for decisions regarding further remediation.

The topics presented in this document are outlined below. Each Section begins with a short summary of its contents.

_	Section	Subject
-	2	Site Description & Facility History
	3	Local Geology
	4	Local Hydrology
	5	Site Groundwater Investigation
	6	Chemicals of Concern
	7	Contaminant Transport Analysis
	8	Current & Future Land Use
	9	Risk Characterization
	10	Conclusions

References, Tables, Figures, and Appendices are found behind the text in separate sections.

2.0 SITE DESCRIPTION AND FACILITY HISTORY

UNISON Transformer Services operated a transformer decontamination and reclamation facility from 1988 to 1991 in the Fairfax Industrial District of Kansas City, Kansas. As part of closure, approximately 2200 tons of PCB contaminated soil and asphalt from the building and parking lot areas were excavated and landfilled. PCB contaminated metal from various equipment and ductwork were removed and landfilled. Contaminated soil from the northwest corner of the building has been recently excavated to groundwater level and removed. TCE and PCB contamination from this area of the facility is believed to have occurred long before UNISON operated the facility.

This section will outline the history of operations at the facility and provide a synopsis of the remedial activities that have taken place and the present situation.

2.1 Setting

UNISON Transformer Services, Inc. (hereafter referred to as UNISON) has leased the facility at 3126 Brinkerhoff Road since 1988. Located in the Fairfax District of Kansas City, Kansas, the facility is surrounded by industry in all directions. Figure 2-1 indicates the location of the facility in the Fairfax District. To the south, the facility shares the same building structure with a segregated warehouse operated by Central Solutions, Inc. The parking lot and loading area borders Central Solutions' production facilities and offices to the north. Brinkerhoff Road and a Union Pacific Railroad spur border the property to the east and west, respectively. A to-scale map of the facility is shown in Figure 2-2. The property is roughly 32,300 sqft (190'x170') or about 0.75 acres.

2.2 History of Operations

Resulting from chemical handling practices prior to UNISON operation, PCB and TCE contamination at the site is believed to have occurred between 1983 and 1986.

The main business of the facility for the last decade has been decontaminating electrical transformers which used fluids containing polychlorinated biphenyls (PCB) and reclaiming the

clean metal parts. Trichloroethylene (TCE) was used as a degreasing solvent to clean the metal parts. Several thousand gallons of TCE were used, recycled, and stored on site. TCE was stored in a large above ground tank just inside the northwest corner of the building and was used in a "triple-sump" degreasing operation in the present "pit" area. The triple-sump involved the transformer parts being washed in three consecutively cleaner solutions of TCE. After flowing through the sump area, TCE was redistilled.

From 1983 to 1986, before UNISON was involved with the operations, reported deficiencies in handling of TCE resulted in spills and leaks. Transformer decontamination started with pilot operations in early 1983, and full-scale operations in June 1984, when an interim permit to handle PCB was granted to EIES (Environmental International Electrical Services, Inc.). In August 1986, EIES overhauled and improved its chemical handling process and storage apparatus. The inside and outside of the facility were "surface" cleaned for PCB, new containment systems were installed, and mass balances on TCE were done regularly. After August 1986, it is unlikely that TCE was introduced to the subsurface.

Distillation units were installed in April 1988 to recover and recycle TCE in the process. UNISON obtained operating rights from EIES in 1988 and completed the commercial repermitting process in late 1989. The facility operated on a limited basis until 1991, when it was shut down and decommissioned. No known spill or leak incidents were reported during the period of UNISON operation. Site clean-up activities relating to closure have been on-going since 1992.

2.3 Remediation by UNISON

UNISON originally contracted with USPCI for soil removal and disposal and started remediation in December 1992, after the unit was shut down. PCB contaminated concrete from the floor of the building and the underlying nominal 2 feet of soils were excavated and disposed of in a TSCA permitted landfill. The walls and ceiling were sandblasted, and the residual dust was vacuumed from the interior surfaces. Approximately 2200 tons of PCB contaminated soil and asphalt from the parking lot areas were excavated and landfilled. PCB contaminated metal from various equipment and ductwork were removed and landfilled.

During interior soil excavation in early 1993, an area of soil below the floor, approximately 50 ft by 10 ft, was determined to be contaminated by TCE as well as PCB. The area is located along

the west wall, in the northwest corner of the building. Soil sampling indicated contamination to 10 ft below the footer of the building. UNISON contracted with Westinghouse Remediation Services, Inc., to excavate the PCB contaminated soil, and treat the soil *ex-situ* to remove TCE to acceptable levels prior to landfilling. To preserve the physical and structural integrity of the west wall of the facility, soil was excavated in "cells", down to a final nominal depth of 13 ft below the building footer. Figure 2-2 indicates the location of the contaminated area as well as the size and placement of the cells.

Upon excavation, the soil was treated by vapor extraction systems in truck roll-off bins to remove the TCE below a level of 5.6 mg/kg in the soil. Upon meeting the criteria, the soil was disposed of in a RCRA/TSCA landfill. Only one soil batch did not meet the TCE standard within the 90 days allowed for temporary storage without a RCRA permit, and it was incinerated prior to landfilling.

The late summer Midwest flood of 1993 temporarily delayed soil removal procedures. At the time, an initial cell was excavated to approximately 6 ft below the footer of the building, and groundwater was observed to a height of 4 ft below the footer. The descent of the groundwater was predicted, based on site observations and at the Missouri River, in order to reinitiate safe excavation of the cells. Figure 2-3 shows the elevation of the groundwater with time, along with a similar plot of the level of the Missouri river.

Cell excavation finally extended to a nominal 13 ft below the footer (approx. 19 ft below typical grade surface). Westinghouse consulted with Alpha-Omega Geotech Inc., who strongly advised that excavation beyond 13 ft. below the footer be avoided and that efforts be taken to stabilize the foundation. Alpha-Omega stated that digging near the groundwater level could cause the immediate failure of the cell walls, undermining the structural integrity of the footer of the facility. The letter from Alpha-Omega is included as Appendix A.

In March 1994, three cells were sampled for both PCB and TCE in accordance with sampling protocol and clean-up criteria. Analyses by EPA and UNISON, revealed that the cells did not meet the PCB criteria (<10 mg/kg - 95% UCL) but did pass the TCE criteria (<60 mg/kg - 95% UCL). Table 2-1 shows the results of that sampling event. UNISON was faced with the immediate decision of attempting further soil removal (due to PCB), which was not advised due to both structural and technical difficulties, or leaving a small amount of contaminated soil, some 20 ft below the original concrete floor. Limited sampling indicated that PCB contamination extended no more than 18 additional inches. UNISON elected to backfill and stabilize the cell

openings, in order to avoid the imminent risk of a safety incident or property damage, and leave a small amount of PCB contaminated soil buried in place beneath 19 ft of clean soil fill and a new concrete floor.

2.4 Present Course of Action

Contaminated soil removal has been completed in the pit area to a nominal depth of 13 ft below the building footer. Some complications have developed underneath the northwest corner of the building. The structure needed to be temporarily supported by a cantilevered steel beam during excavation, and the clean fill material eroded to some extent, allowing some settling of the building structure. Actions are underway to secure and correct this situation.

Based on historical and physical evidence, UNISON believes the pit area to be the sole source of TCE related contamination. TCE traveled downward and contaminated the groundwater in a localized area directly beneath the building. Two shallow groundwater wells (PZ-1,-2) were installed in January 1994, to determine if there was a spread of TCE. TCE was found directly north of the building (170 mg/L) and at much lower levels near the southeast corner (0.340 mg/L). These findings prompted a May 1994, groundwater investigation, which is discussed fully in Section 5.0.

This risk assessment is intended to answer questions on the health and environmental impacts that result from: 1) leaving a small quantity of contaminated soil in the subsurface; and 2) the migration of any PCB and TCE in the groundwater.

3.0 LOCAL GEOLOGY

The Brinkerhoff Road site is generally situated in the Central Lowlands Physiographic Province and more specifically in the Missouri River floodplain. The topography is dominated by the floodplains and bluffs of the Missouri River. The first bedrock is the Pleasanton Group, which is made mostly of shales and about 70 ft thick at the site, rendering it relatively impermeable. No faults are found in the area. The soils at the site are alluvial in nature and can be broken down into three regions:

- Upper 40% of vertical profile silt, clay, and very fine sand;
- Middle 40% medium sand with interlayers of silt and coarse sand; and
- Lower 20% coarse sand and gravel.

This section discusses the regional and local physiology, bedrock, and soils.

3.1 Regional Physiography

The Kansas City region is located in the Central Lowlands Physiographic Province and lies on the boundary between two sub-provinces: the Dissected Till Plains Subprovince to the north and the Osage (scarped) Plains Subprovince to the south (Figure 3-1). The boundary between the two sub-provinces is approximately either side of a general east-west line formed by the Kansas River (south of the Brinkerhoff Road site) and the Missouri River, east of its junction with the Kansas River (Jewett, et. al.,1965).

The Dissected Till Plains Subprovince consists of rolling topography formed on the earliest glacial (Kansas and Nebraskan) tills and outwash, as well as later loess (post-glacial aeolian silt) deposits. Sufficient time has elapsed that later post-glacial stream activity has dissected and eroded the surface, and few glacial features remain. Where severe erosion has occurred in the uplands along major rivers, the landscape may be extremely hilly, with exposed bedrock and sharp relief. The Dissected Till Plains generally dominate the topography north of the Kansas River in the immediate site region (Zavesky and Boatwright, 1977).

The Osage Plains Subprovince consists of low relief, gently rolling hills and plains formed by stream erosion of interbedded Pennsylvanian shale and limestone bedrock. The hills are shallow escarpments (cuestas) of resistant limestone while the wide valleys are softer shale. The width of the cuestas and valleys is controlled by the thickness of the individual rock units and the angle of

dip. The Osage Plains form the dominant topography south of the Kansas River in the region.

The Missouri River floodplain exhibits alluvial topographical features distinct from those of the Dissected Till Plains and Osage Plains Subprovinces. The Missouri River is in a mature stage of development with a wide floodplain, long meander belts, and related features such as abandoned channels, meander scars, and natural levees. The Missouri has an overwide floodplain and the alluvial deposits are unusually thick compared to expected values based on volumes of water and the size of the existing channel. The size and depth of the alluvium filled valley apparently is due to the large volumes of glacial meltwaters and sediments carried by the pre- and post-glacial ancestral Missouri River (Simms, 1967).

The Missouri River, and the ancestral Kaw River, appear to have occupied nearly the same valley since the latest regional glaciation. The Kaw River valley contains thick, near-surface glacial lodgement tills south of the site. Outwash and tills have been found beneath the Missouri River alluvium indicating the pre-glacial age of the river system. The Kansan glacial advance(s) apparently forced the pre-glacial Missouri system to flow south of the present bluffs and then easterly, before it rejoined the main channel in eastern Jackson County, Missouri. A north-south trending glacial debris filled valley crosses Kansas City and Kansas uplands, just west of the site. In the post-glacial period, the Missouri River reoccupied its former (and present-day) valley (O'Connor and Fowler, 1963; Jewett, et. al., 1965).

3.2 Regional Soils

Soils in the region vary greatly in composition and origin. An east-west line paralleling the Kansas and Missouri River valleys marks the division between the physiographic subprovinces and forms a rough division between the glacially-derived regional soils to the north, and the predominantly residual and colluvial soils to the south (Figure 3-1). Glacial tills and outwash materials are common in the uplands north of the Missouri River and immediately south of the Brinkerhoff Road site (Figure 3-2), whereas residual soils dominate south of the Kansas River. The two rivers generally have thick alluvial deposits resting either on bedrock (Figure 3-3) or on glacial lodgement till or outwash. Loess occurs as thick deposits, veneering the bluffs adjacent to the rivers, but it tends to thin rapidly away from the rivers.

The glacial deposits north of the Kansas River vary from clay lodgement tills, predominantly low and highly plastic clays (CH, CL), to coarse sand-gravel (SP, GP) outwash materials. These

deposits originated during the Kansan and possibly the Nebraskan glaciation. Later glaciers did not advance as far south as the Kansas City area. Glacial tills and outwash materials have been encountered in deep borings in portions of the Missouri and the Kansas River valleys, and at sites south of the Kansas River; thus, indicating isolated lobes of the Kansan glaciers probably surged south of the ancestral river valley (Feder, 1974; O'Connor, 1971).

Alluvium is present in the major river floodplains in the region. The alluvium generally consists of cohesive highly plastic clay and low plastic clay and silt (CL,CH,ML) and very fine-grained granular silty-sand (SM) soils overlying progressively coarser deposits (SP,SW) (Figure 3-3). In most areas, extremely coarse-grained materials (coarse sand and gravel, and boulders deposits designated as SP, GP, GW) are found immediately above bedrock. In smaller streams and rivers, residual low and highly plastic clay (CL, CH) soils often are found between the alluvial or colluvial valley fill material (SP,GP) and the bedrock (KCK,1986; Layne-Western,1984).

Loesses (low plastic clayey silts - ML) and weathered loesses (CL/ML) were formed from windblown silts carried from the front of the glaciers, as they melted, or from the sediment choked river valleys. The silts were deposited by the wind in thick layers along the bluffs closest to the rivers and in progressively thinner deposits further away from the sources (Zavesky and Boatwright, 1977).

3.3 Regional Bedrock

The near-surface and exposed bedrock units in the site region are part of the lower and middle Pennsylvanian-aged rocks. The regional bedrock pattern is controlled by the Forest City Basin, centered about 50 miles north-northwest of the site (USACE,1979). General regional dips are shallow (<1°) towards the center of the basin and away from the immediate Kansas City area, in a northward direction; thus, progressively younger rocks are exposed towards the center of the basin.

Mississippian and older Ordovician and Cambrian rocks lie below the Pennsylvanian. The Mississippian outcrops well to the south of the near-site region in southwestern Missouri and southeastern Kansas. These are oldest exposed rocks in the region. The Mississippian rocks are chiefly dolomites and limestone with some major chert zones. The Cambro-Ordovician units are predominantly dolomite with some sandstone and occasional shale beds (O'Connor,1971).

The Pennsylvanian rocks in the region are divided into eight geologic Groups. The regional Groups, in order from youngest to the oldest, are the Shawnee, Douglas, Lansing, Pedee, Kansas City, Pleasanton, Marmaton, and the Cherokee. A generalized regional stratigraphic column for the Pennsylvanian is presented in Table 3-1, and a near-region geologic map is presented in Figure 3-4.

Lansing, Douglas, and Kansas City Group rocks dominate the uplands north and south of the River in the area of the site. Portions of the Kansas City, Pleasanton, and Marmaton Group rocks form the bedrock beneath the glacial and thick alluvial deposits in the river valleys. Progressively older Pleasanton, Marmaton, and Cherokee Group rocks are exposed in the uplands in the region to the east and southeast, whereas the progressively younger rocks are exposed to the north and west of the site.

The Cherokee and Marmaton Groups are present in the subsurface throughout the region, but outcrop only well south and east of the area. The Cherokee Group consists of a succession of thick clastic sediments, predominantly sandstone and shales with some interbedded siltstones, underclays and coal beds. The overlying Marmaton contains a series of interbedded shales, sandstones, underclays, coal, and limestones.

The Pleasanton Group outcrops in the southern and eastern sections of the region and forms the bedrock surface under much of the Missouri River floodplain, east of the site. With a few exceptions, the bulk of the Pleasanton strata are clastic materials. Shales and claystones dominate the sequence in the immediate site area, but sandstones are found to the east of Kansas City. Minor limestones occur throughout the sequence. The Pleasanton Group is over 120 ft thick in portions of the region, but thins rapidly north and west of the site. (Figure 3-4).

The Lansing and Kansas City Groups form the bedrock surface on the bluffs and in the uplands, both north and south of the Missouri River, in the immediate site area. The Kansas City Group consists of a series of interbedded shales and limestones forming repetitive cyclic sequences (O'Connor, 1971). Limestones dominate the lower portion, and shales form the bulk of the upper half of the group. Occasional sandstone units are found within or replacing the shales near the top of the sequence. The Kansas City Group varies from 250 to 280 ft thick in the immediate site area (O'Connor, 1971). The Lansing Group has lithologies and sequences similar to the Kansas City Group. The Lansing consists of cyclic sequences of limestones and shales with some coarse sandstones and conglomerates. The Lansing outcrop zone is to the west of the central Kansas City area (Figure 3-4).

Pedee, Douglas, and Shawnee Groups occur above the Lansing. These units form the upland bedrock surface to the west and north of the site. The Shawnee Group consists of cyclic sequences of limestones and shales with some coarse sandstones and conglomerates similar to the Lansing and Kansas City Groups. The Pedee and Douglas are predominantly bedded shales and siltstones with occasional coarse clastics (including channel sands) and some limestones. In the immediate area, the Lansing and Pedee Groups are each some 60 feet thick, the Douglas Group ranges from 110 to 150 feet, and the Shawnee Group is approximately 250 feet thick.

Faults are relatively rare in the Kansas City region but are present in the pre-Pennsylvanian rocks, well below the uppermost bedrock at the site. Complex faulting, associated with down-dropped blocks from pre-Pennsylvanian cavern collapse, is present in the southern part of the Kansas City region, but none have been reported in the immediate site area. Jointing is especially well developed in the numerous limestone units in the stratigraphic sequence and, to a lesser degree, in the shales of the Pleasanton and Kansas City Groups. The majority of joints tend to be near vertical, and strikes vary from northeast to northwest, across the region.

3.4 Site and Near-Site Bedrock

A major channel of the ancestral Missouri River flowed west to east, crossing the Fairfax District south of the present Missouri River channel in a concave crescent shaped trough. A bedrock topographic map is presented in Figure 3-5 (Simms, 1967; and Neyer, et. al., 1985).

The bedrock surface is wide with low relief, at a mean elevation of approximately 650 ft msl with steep side walls at the edge of the valley. Imposed on this bedrock surface is a deeper channel representing the ancestral Missouri River, which eroded a deep, moderately steep walled trough in the bedrock surface to elevations on the order of 600 ft msl. The boulder filled portion of this channel is at about 640 ft msl and is roughly 3,000 feet wide (Figure 3-6) (Simms,1987). The center of this channel is approximately 2,000 feet northeast of the site at its nearest approach. The Kansas City Group forms the valley walls of the present Missouri River, and the Pleasanton Group forms most of the wide, low-relief bedrock surface and the ancestral Missouri channel (Simms,1967; and Jewett, et. al.,1965).

The bedrock beneath the site is composed predominantly of Pleasanton Group rocks (Figure 3-4). Determination of bedrock formations and conditions in the site and near-site area are based on

bedrock elevations derived by Simms (1967); Neyer, et. al. (1985); boring logs presented in the bid specifications for the City of Kansas City, Kansas (KCK, 1986); regional compilations by the Corps of Engineers (1979), and the Trans-Missouri Tunnel to the east of the Missouri River (Woodward-Clyde, 1990a and b).

Minor limestones and sandstone strata occur throughout the Pleasanton; however, shales and claystones dominate in the immediate site area. Borings penetrating the bedrock in the Fairfax District indicate Pleasanton rocks in the near-site vicinity are gray shales or sandy shales with some pure sandstone interbeds. A boring 300 ft west of the site intersected very hard gray limestone at the bedrock surface at 640 ft msl. A boring 600 ft to the northwest had light blue gray shale and sandstone at 641 ft msl, and a boring 600 ft southwest of the site had gray, carbon bearing shale at 646 ft msl. (Layne Western, 1984; KCK, 1986; and Fishel, 1948)

Based on nearby bedrock elevations and a 95 ft thickness of the Pleasanton Group, about 25 ft of the Group was eroded by the ancestral Missouri River, and 70 ft thus remain beneath the site (Hasan, et.al.,1988; Woodward-Clyde,1990a,b). Logging conducted in North Kansas City, east of the Missouri River, indicate that the Pleasanton Group rocks are relatively impermeable. Groundwater in this strata is carried primarily through joints in the limestone and coarse clastics or along bedding planes between distinct lithologic units (SCS, 1990).

3.5 Site and Near-Site Alluvial Sediments

The Missouri River deposits rather than erodes sediment, and it flows in a valley that is far larger than would be expected for a river this size. Deep, early Pleistocene channels cut into the bedrock were filled by glacial deposits and covered by Missouri River alluvium (Simms, 1967; O'Connor, 1971; Jewett, et. al., 1965). The deep ancestral Missouri River channel flowed in an easterly direction to a point northeast of the site and then turned south to its junction with the ancestral Kansas (Kaw) River (Simms, 1967). This ancestral channel cut across the central portion of the Fairfax District within 600 feet of the site.

The division of Missouri River sediments into three zones applies also to the Fairfax District (KCK,1986):

- An upper cohesive soils zone of silt, clay, and very fine silty sand;
- A middle zone of fine to medium sand; and
- A lower coarse sand and gravel zone.

The near-site area appears to have greater vertical variability and substantially more fine grained sediments at depth than were recognized elsewhere in the Missouri River alluvium (Crabtree and Malone, 1984; SCS Engineers, 1990).

Poorly graded sediments are most common in the Fairfax District. Approximately 20% of the vertical sediment profile is coarse-grained sands and gravel; 40% is medium-to coarse-grained sands with some fine fraction; and 25% is very fine grained interbedded sands and silts. The remaining 15% consists of surficial deposits of silts, clays and very fine-grained silty sands accompanied by fill (KCK,1986; Layne-Western,1984; Fishel,1948).

The uppermost zone of the natural alluvial profile is obscured by construction, excavation, fill placement activities at the site. It consists of intermixed clays and silts with some very fine-grained silty-sand lenses (ML, CL and SM). The clays are generally low to medium plastic clays, grading into clayey or sandy silts and very fine silty sands. Fill and concrete has been placed over the natural soils to attain present grades. On site, the upper zone varied in thickness from 12 to 18 ft and is reported to vary from 5 to 35 ft elsewhere in the Fairfax District (Fishel,1948; Corps of Engineers,1979; KCK,1986; and Layne-Western,1984). No increase in grain size with depth was noted at Boring S-1 at the site, although such grain size increases are common in the Fairfax District (Jewett, et. al.,1965; Fishel,1948).

The poorly graded middle zone materials at the site contain clay seams, fine sand and gravel lenses, and organic residue (lignite or decayed wood) deposited in the sand. These features are common in the sand deposits throughout the near-site Fairfax District. The medium grained sands at S-1 went from 12 to 58 ft deep. Fine granular material and medium sands comprised more than 50% of the sequence at S-1. This percentage is somewhat higher than others noted in Fairfax District boring logs (Fishel, 1948; Corps of Engineers, 1979; KCK, 1986).

The lower alluvial zone, consisting of coarse sand with fine to coarse gravels, was encountered only at the base of Boring S-1 at a depth of 58 ft. The gravel content of the lower zone typically ranged from ranges from 20 to 50 percent in Fairfax District.

4.0 LOCAL HYDROLOGY

Three basic types of aquifers are present in and around the Kansas City area: upland soil aquifers; shallow and deep bedrock aquifers; and alluvial aquifers. Of these, the alluvial aquifer system of the Missouri and Kansas rivers is the most important, as it has the highest yields, transmissivities, and storativities of any regional aquifers. The groundwater beneath the Brinkerhoff Road facility is part of the Missouri River alluvial aquifer. The conservative estimates of local aquifer properties used in the fate and transport modeling have a basis in the data collected and the groundwater models developed at or near the Fairfax District.

This section discusses the regional and local aquifers and gives estimates of aquifer properties at the Brinkerhoff Road site, based on results from other local studies.

4.1 Regional Hydrogeology

Several diverse groundwater regimes are present at the regional level in the site area. The groundwater domains include generally perched upland soil aquifers, multiple bedrock aquifers, and the alluvial aquifers of the Missouri and Kansas Rivers and their tributaries (O'Connor, 1973; SCS, 1990).

4.1.1 Regional Upland Soil Aquifers

Upland soil aquifers are generally found in the glacial tills and outwash materials in the region north of the Missouri River and in the colluvial and residual upland soils south of the river. These soil aquifers are mostly thin, perched, unconfined aquifers with only occasional confining upper boundaries. Groundwater yield in the upland aquifers is generally low, on the order of a few gallons per minute.

4.1.2 Regional Bedrock Aquifers

Groundwater in the regional bedrock occurs in limited shallow zones and deeper regional aquifers. The uppermost bedrock aquifers are fresh water shallow perched zones and unconfined

or semi-confined zones. The deep or regional bedrock aquifers occur in the Pennsylvanian and Mississippian rocks and in multiple confined zones in the Ordovician and Cambrian rocks below the Mississippian. Regional bedrock aquifer characteristics vary considerably. Generalized bedrock aquifer water quality and yields are presented in Table 4-1.

Typically, shallow bedrock aquifers are found in the clastic strata or in the fractured limestones of the Kansas City or overlying Shawnee, Douglas and Lansing Groups, or in the granular sedimentary rocks of the Pleasanton, or carbonate rocks in the lower Pennsylvanian, underlying the Kansas City Group (O'Connor,1971). These perched bedrock aquifers are limited in area and in thickness and generally have lower confining shale layers. Multiple perched aquifers may be present in vertical profiles through the lower (clastic) portions of the lower Douglas Group, in the Pedee Group channel sandstones, and in the lower portion of the Kansas City Group. Recharge to these perched bedrock aquifers is from upland rivers and streams, precipitation infiltration through the overlying soils, or leakage from shallow soil or bedrock aquifers to the lower perched zones. Shallow aquifer groundwater generally is potable, but exploitation for other than domestic or limited agricultural use has been rare in the region because of extremely low recharge rates.

In western Missouri and eastern Kansas the Mississippian and Cambrian-Ordovician deeper aquifers tend to be saline, i.e., > 1000 mg/L total dissolved solids (TDS). Missouri wells in the immediate site region ranged from 16,000 to 41,000 mg/L TDS. Similarly, Missouri wells in the Atokan Series and Cherokee Group (lower Pennsylvanian) in the site region ranged from 15,000 to 31,000 mg/L TDS. In contrast, dissolved solids for shallow wells in the middle Pennsylvanian in Johnson County, southeast of the site, ranged from 119 to nearly 5,100 mg/L TDS.

Near-site aquifer characteristic data for the bedrock aquifers have rarely been published and are not generally available. Regional data have been complied from published and unpublished U.S. Geological Survey (USGS) and Kansas Geological Survey (KGS) files or private sources.

4.1.3 Regional Alluvial Aquifers

The alluvial aquifers are the major groundwater sources in this portion of east-central Kansas and west-central Missouri. The alluvial aquifers are contained in the unconsolidated sand and gravel materials deposits comprising the past and present alluvial floodplains in the region (Figure 3-3). The alluvial aquifers are part of the regional river system composed of the Missouri River and its major tributary, the Kansas River. In the near-site area the lower reaches of Jersey, Nearman,

Line, and Eddy Creeks form part of the Missouri River alluvial aquifer but are not exploited as groundwater sources.

The sediments comprising the alluvial aquifer in the Missouri River floodplain in the Kansas City area consist of three generalized layers. The upper portion consists of cohesive and fine grained granular soils, formed primarily as overbank flood deposits. The middle section is made of well graded sand, with occasional interlayering of coarse granular or cohesive materials, reflecting previous river flowpaths. The lower portion of the sequence is a mixture of coarse sand, gravel, and boulders. A general increase in alluvium grain size with depth is well documented in published and unpublished data.

Groundwater is generally unconfined in the medium to coarse grained alluvial aquifers. However, the uppermost unit, consisting of lower permeability very fine-grained or cohesive alluvial materials, may produce semi-confined conditions. Studies of aquifer relations in the floodplain in the near-site region indicate a lag time in dewatering or saturation of the surficial materials compared to the deeper coarse sediments during rising or falling river stages. Regionally, water levels in the alluvium average from 5 to 25 feet below ground level. The saturated thickness typically varies from approximately 70 to 110 feet.

Groundwater flow is controlled by groundwater gradient and hydraulic conductivity (GeoTrans, 1985). Missouri River levels affect local groundwater flow directions (Geotrans, 1985; SCS, 1990). High river levels, generally in the spring, force inland groundwater flow, and low river levels promote groundwater flow riverward. Groundwater flow tends to cross broad river meanders in a general downstream direction.

Alluvium aquifer recharge is from the Missouri River and its tributaries, infiltration from precipitation, and streams or impoundments on the floodplain. Recharge from the bedrock beneath the alluvium is considered to be minimal in the near site region, but bedrock discharge from the bluffs and valley walls is a large source of recharge during falling river stages.

4.1.4 Regional Alluvial Aquifer Characteristics

The alluvial aquifers have high hydraulic conductivities and transmissivities compared to bedrock and upland soil aquifers. Regional alluvial aquifer characteristics vary considerably between the

river systems and within each system. Generalized alluvial aquifer characteristics are summarized in Table 4-2.

Pump tests approximately 9 miles downstream at the former Riverfront Landfill (SCS, 1990) indicate a specific yield of 0.08 and transmissivities between 29,400 and 30,800 ft²/day. Hydraulic conductivities varied from 3.6x10⁻³ to 1.6x10⁻¹ cm/sec. Foreman (1979) measured hydraulic conductivities near Columbia, Missouri, from 1.97x10⁻¹ to 3.87x10⁻¹ cm/sec. Crabtree and Malone (1984) indicated that hydraulic conductivity results at the Conservation Chemical Company (CCC) site, approximately 12 miles downriver of the site, correlated well with sediment grain size, as indicated in Table 3-4. They and others (Moraes, 1971, cited by Zatezalo, 1977) found a nearly exponential increase in hydraulic conductivity with the downward coarsening of the alluvial sediment. Geotrans (1985) found that deeper layers have hydraulic conductivity values at least an order of magnitude greater than shallow layers.

Regional data indicate transmissivity values ranging from 6,700 to more than $100,000 \text{ ft}^2/\text{day}$. Storage coefficients range from 2.2×10^{-4} to 0.27 (unitless). Local groundwater hydraulic gradients are very low, on the order of 0.005 (Crabtree and Malone,1984; Geotrans,1985; Reed and Burnett,1985). Foreman (1979) calculated the ratio of vertical to horizontal conductivity to be between 2.6×10^{-4} and 7.2×10^{-4} .

Geotrans (1985) modeled the alluvial aquifer for the CCC project, downriver from the Brinkerhoff Road site. A two layer baseline model was developed for input into detailed simulation models. The upper aquifer zone (K₁) was modeled as 30 feet thick with a hydraulic conductivity of 1x10⁻² cm/sec. The lower zone (K₂) was considered to be 60 feet thick with an hydraulic conductivity of 1 cm/sec. The model used a 0.4 vertical conductivity to horizontal conductivity ratio (K_V:K_H) and a storage coefficient of 0.2. A model derived for the Fairfax District by Neyer, Tiseo, and Hindo LTD. (Neyer, et. al.,1985) found storativity and infiltration values similar to others (Geotrans,1985; Crabtree and Malone,1984; Foreman,1979; and SCS,1990). The Neyer transmissivity values (> 300,000 gpd/ft) were substantially greater than those derived by Geotrans (1985), SCS (1990), and Foreman (1979) but were in accordance with Crabtree and Malone (1984). The Neyer model components are used as the basis for the aquifer characterization of the site area.

4.2 Site and Near-Site Alluvial Aquifer

The alluvial aquifers of the Missouri and Kansas Rivers are the major groundwater sources in this portion of east-central Kansas and west-central Missouri. They are contained in the unconsolidated sand and gravel deposits comprising the past and present alluvial floodplains in the region. The upper portion of the sequence consists of cohesive and fine-grained granular soils. The middle section consists of poorly graded sands, with occasional interlayering of coarse granular materials. The lower portion of the sequence typically is a mixture of coarse sand and gravel. A general increase in grain size with depth is well documented.

Regionally, water levels in the alluvium will average from 5 to 25 feet below ground level; the average saturated thickness typically varies from approximately 70 to 110 feet. The Missouri River generally dominates local groundwater flow, as described in Section 4.1.

Regional alluvial aquifers have high hydraulic conductivities and transmissivities compared to bedrock aquifers. The site/near-site aquifer characteristics are generally consistent with regional characteristics and are summarized in Table 4-3. Values applicable to the Brinkerhoff Road site are presented in Table 4-4. The data are based on the findings of Neyer, et.al. (1985), Fishel (1948), Geotrans (1985), and Feder (1974).

<u>Bedrock Elevation</u>: Bedrock elevations in the Fairfax District vary considerably due to the presence of a deep ancestral river valley. The bedrock elevation directly beneath the site is estimated to be approximately 645 ft msl, based on the data presented by Neyer et. al. (1985).

<u>Alluvium Thickness</u>: The bedrock elevation dictates the alluvium thickness in the relatively flat Fairfax District (65-145 ft, 100 ft at the site).

<u>Saturated Aquifer Thickness:</u> Depth of saturation is dependent on the alluvium thickness and the Missouri River stage. Average thicknesses in the Fairfax District (43-123 feet) and at the site (80 feet) are based on historical river levels and bedrock elevations. (Katzman and Luce, 1991; SCS, 1990; Crabtree and Malone, 1985)

<u>Transmissivity:</u> Thirty two wells in the Fairfax District have a wide range of transmissivities, from 77,000 to 825,000 gpd/ft, with a median value of approximately 400,000 gpd/ft. Only one well was found less than 200,000 gpd/ft. The transmissivity values for the Fairfax District are

much higher than other local Missouri River alluvium (Geotrans,1985; SCS,1990; Foreman,1979).

<u>Coefficient of Storage</u>: An average storage coefficient of 0.15, based on the modeling done by Neyer, et.al. (1985), is assumed to adequately represent the Fairfax District. This value is much greater than other scenarios for the regional alluvial aquifer, in which coefficient values ranged from $1.0x10^{-3}$ to $2.2x10^{-4}$.

<u>Yield:</u> The range in yield for 22 wells in the Fairfax District varied from 80 to 1580 gpm, with an average and median of about 980 gpm. (Fishel,1948) Thus, an estimated yield of 1000 gpm is reasonable for a well at the site.

<u>Specific Capacity:</u> Values for the Fairfax District ranged from 13 to 375 gpm/ft. The average of 60 gpm/ft is a reasonable estimate of specific capacity at the site.

Permeability: Various near-site area values range from 1072 to 7313 gpd/ft² (Neyer,et.al.,1985). Values from fine soils (SM, SP-SM) were estimated in the range of 200 to 800 gpd/ft², and granular soils (SP) varied from 570 to 4100 gpd/ft². A conservative permeability estimate of 5000 gpd/ft² was selected for groundwater modeling at the site, based on an expected transmissivity of 400,000 gpd/ft and an aquifer thickness of 80 ft. A reasonable estimate of the permeability for the fine grained soils (SM/SP-SM) is 600 gpd/ft². The value for the coarse fraction (SP) was calculated as 6,015 gpd/ft².

Hydraulic Conductivity: Few hydraulic conductivity data have been published for the Fairfax District; thus, conductivities are derived from permeability data. Values range from 3.6×10^{-3} to 3.8×10^{-1} cm/sec for the Fairfax District. The average site hydraulic permeability was estimated to be 2.3×10^{-1} cm/sec, based on an average 5000 gpd/ft² permeability. Based on site geology, the hydraulic conductivity for the upper portion (ML, SM) of the aquifer is 2.8×10^{-2} cm/sec, and the value the lower portion (SP, GP) is 2.8×10^{-1} cm/sec. These values are very conservative and much higher than those given by Crabtree and Malone (1984). (Table 4-5)

5.0 SITE GROUNDWATER INVESTIGATION

The groundwater quality at the Brinkerhoff Road site was investigated for chemical contamination after PCB and TCE contaminated soils were found to go to the water table. The investigation consisted of:

- 1 exploratory boring to determine soil stratigraphy;
- 4 piezometers to give a preliminary indication of groundwater gradient; and
- 6 monitoring wells to determine chemical distribution.

Findings indicate that TCE has dispersed from the source to the property boundaries, primarily eastward, in the direction of groundwater flow. The measured groundwater gradient is 4.0 feet per mile (0.076 %). Within 120 ft of the source, TCE concentrations diminish to approximately 1% of the highest values. TCE is being transformed to 1,2-DCE, which was also detected in several samples, by reductive dehalogenation. The observed detection of PCB in the groundwater is not conclusive, as the samples were not filtered prior to analysis, and the small concentrations (max. 0.023 mg/L) may be due to soil particles with sorbed PCB in the sample water. Decreased chemical concentrations in the deeper wells indicate little vertical migration of these analytes. No free phase chemical was found.

This section details the procedures of the site groundwater investigation and presents the results of groundwater analyses.

5.1 Site Investigation

Hydrogeologic field investigations were conducted by ERM - Rocky Mountain, Inc., in April and May of 1994. Field activities included mobilization, site layout, drilling, installation and sampling of groundwater monitoring wells, and surveying of the property. Existing data were reviewed, and an abbreviated Health and Safety Plan (HSP) was prepared prior to field activity. The HSP presented a site description, emergency information and procedures, and specific and general field safety procedures. A copy of this plan was kept at the site throughout the field activities. ERM and subcontractor personnel were required to read and sign the HSP prior to the field activities.

The first phase of field work was drilling one 60 ft deep stratigraphic boring (S-1) to determine soil stratigraphy and installing two additional piezometers (PZ-3,-4) to determine local groundwater head gradient. Six monitoring wells were installed in the second phase to determine

the distribution of chemicals in the subsurface. Groundwater from the six wells and four piezometers were sampled and analyzed. Also, all boring and well locations and elevations were surveyed, and these data were incorporated into the site maps. Well and piezometer locations are indicated in Figure 2-2.

5.1.1 Borings, Monitoring Well and Piezometer Installation

The initial phase of field activities included the drilling of one stratigraphic boring and the installation of two piezometers.

A 60-foot deep stratigraphic boring (S-1) was drilled near the west-central edge of the site. This boring was initially advanced with a 3-7/8 inch fishtail bit using a bentonite-water slurry. The boring was drilled with a Gardner-Denver D-50 truck mounted drill rig. Upon reaching a depth of 30 ft the boring was reamed and a 4 inch ID steel casing and bentonite seal were installed to seal the upper portion of the boring from the lower portion. Drilling resumed through the 30-foot steel casing.

Split spoon soil samples were obtained using Standard Penetration Test (SPT) methods. Samples were taken on 24-inch centers to 22 ft, and on 18-inch centers below that depth. They were then preserved in glass jars and labeled. The boring was logged using the Unified Soil Classification (USC) System descriptions of the samples, and observations of drill rig advancement, downpressure, and drilling characteristics. Boring logs are presented in Appendix B.

A small portion of each sample was temporarily placed in a plastic bag and labeled. A photo-ionization device (PID) was used to measure contaminant levels from each split spoon. The PID was used on the head space in each of the plastic bag samples and on the borehole and breathing space zones for Health and Safety monitoring.

Two (2) borings were drilled to depths of 30 feet for the installation of piezometers. One boring was in the building at the southwest corner of the site and the second in the northeast corner of the site, along the fence line. The borings were drilled with a CME-55 mounted on a Ford tractor using 4-34 inch ID hollow stem augers. A Teflon plug was used to prevent sands from entering the hollow stem augers during the drilling process. These borings were advanced to approximately 10 feet below the unconfined groundwater surface.

Piezometers (PZ-3 and PZ-4) were installed in each of these two borings. Since the soil materials were considered unstable, the piezometers were constructed through the inside of the hollow stem augers. The piezometers were made of 2 inch ID, Schedule 40, threaded and flush-jointed PVC pipe. The lower 10 feet of each piezometer were screened with 0.01 inch factory-machined slots. A flat cup or well point was attached to the bottom of each piezometer screen. Filter material, consisting of a fine to medium sand, was placed around the well screen using a tremie. The filter pack was installed to approximately 2 ft above the top of the screened interval, and approximately 2 ft of bentonitic clay seal was placed above the filter. Hydrated bentonite chips or pellets were used above the water table. The riser pipe was backfilled with a cement-bentonite slurry to grade. A flush-mounted protective steel locking surface casing was placed around the piezometer riser pipe and a shallow concrete pad was constructed at the surface around the surface casing. Piezometer construction diagrams are presented in Appendix B.

The piezometers were developed by pumping between ten and twenty well volumes of groundwater from each piezometer. Initially, a 2 inch diameter stainless steel bailer was used for purging. Later, a Redi-flow electric pump (1.5 - 2 gpm) was used. All development water was stored in 55-gallon drums, sealed, and labeled. Groundwater levels were measured from the four piezometers to determine the placement of subsequent borings and wells.

Six additional borings were drilled at selected locations within the boundaries of the site. The borings were advanced with a truck-mounted Gardner-Denver D-50 truck mounted drill rig, using hollow stem augers (4 ¾ inch ID) to depths of approximately 30 to 45 ft below the grade. To compensate for hydrostatic pressures forcing flowing (running) sands into the hollow-stem augers, the borings were over-drilled and Teflon knock-out plugs were used. Monitoring wells were installed at all six boring locations. Well installation procedures and materials used were the same as those used for piezometer installation.

Wells MW-14A, MW-13A and MW-13B were finished with flush mounted metal protective surface caps and concrete protective pads. Wells MW-11A, 12A and 12B were placed inside the building in areas that are expected to receive fill material at a later date. These wells were grouted to the present ground surface, and riser pipe was added to extend the wells above ground level. When fill material has been added the riser pipe will be grouted to the new floor surface, and flush mounted protective caps will be installed. Well and piezometer data are summarized in Table 5-1.

Health and safety monitoring during drilling was done through the use of PID monitoring in the

breathing zone, at the well head, on samples, and in the head space of the jarred samples. Field work was accomplished using OSHA Level D personal protective equipment (PPE) with the exception of the work at the MW-13 cluster. High PID readings necessitated upgrading to Level C PPE during drilling and well installation at this cluster.

Well materials were decontaminated by steam cleaning and wrapped in plastic sleeves prior to mobilization to the site. Personnel used disposable plastic gloves during the handling of the well materials. The sampling equipment was decontaminated between samples by scrubbing with an Alconox-distilled water solution and rinsing with distilled water. Decontamination between borings of the hollow-stem augers and sampling equipment was done with high-pressure steam. Drill cuttings and sample materials were collected and placed in disposal units on site for eventual disposal at an hazardous waste landfill. PPE (gloves, Tyvek suits, etc.) were placed in hazardous waste disposal containers in the facility laboratory for later disposal.

5.1.2 Groundwater Level Measurements

Ground water depths were measured at all piezometers and monitoring wells with an electronic water level meter. The high point on each PVC riser pipe was used as a standard reference, as they were surveyed to the nearest 0.01 ft (U.S. Coast and Geodetic Survey datum). Groundwater elevations are listed in Table 5-2.

5.1.3 Groundwater Sampling Procedures

A single round of groundwater samples was obtained from the wells and piezometers to define the extent of contamination by PCB, TCE, and other volatile organic compounds. Pace Laboratories, Inc., of Overland Park, Kansas, was hired to do the analyses.

Each well and piezometer was purged a minimum of 3 well volumes prior to sampling. Well volumes were calculated based on wetted well surface, depth and diameter for each well. A Rediflow electric pump was used to pump purge water at a rate of 0.5 - 2 gpm to either 55-gallon drums or 5-gallon buckets which were emptied into drums. The hoses and pump were decontaminated before use at each well. The purging continued until the physical measurements of the purge water (conductivity, pH and temperature) had stabilized within a 10 percent range. Personnel wore latex surgical gloves when handling the pump and purging equipment.

A stainless steel, two-inch diameter bailer was used to sample each well. The first bailer of water obtained from any given well was used to fill the volatile organic compound vials. No air bubbles were permitted in these vials. Subsequent volumes of water were used to fill the vials for PCB analyses. Appropriate preservatives had been placed in the sample containers by the laboratory prior to shipment to the site.

The stainless steel bailer was decontaminated between each well using alconox-distilled water solution and distilled water rinses. The bailer rope and surgical gloves were discarded between sample points. The decontamination water was collected and disposed of with other decontamination water. PPE and bailer rope were placed in hazardous waste disposal containers in the facility laboratory for later disposal.

One set of laboratory supplied trip blanks and one set of duplicate (blind) samples were analyzed as part of the overall project quality assurance/quality control program. The sample containers were sealed, labeled, and enclosed in self-sealing (zip-lock) polyethylene bags. Sample label designations were based on the well or piezometer number with the exception of the field duplicates (MW-13B) which were given an arbitrary number (MW-15A) in sequence with the total number of wells.

The bagged sample vials were shipped in coolers, packed with vermiculite, and topped with frozen packets of "blue ice". The coolers were sealed, labeled, and transported to Pace Laboratories by ERM personnel. Each cooler was accompanied by a chain-of-custody form signed by the sampler, transporter, and laboratory personnel.

5.2 Site/Near-Site Groundwater Gradient and Flow

Groundwater elevations varied from 727.50 to 727.64 ft msl across the site. The measured groundwater gradient is 4.0 feet per mile (0.076 %) to the east (Figure 5-1). This compares favorably with gradients presented in the literature of four to five feet per mile (SCS 1990; Fishel, 1948).

Groundwater flow data for the Fairfax District are not available. Groundwater velocity was calculated using the Wenzel formula (Fishel, 1948) for fine silty materials and sands:

 $v = [(permeability in g/d/ft^2)(gradient in ft/mi)] \div [(395)(porosity in \%)].$

Velocities ranged from 0.2 ft/day for silts to 2.0 ft/day for sands, based upon conservative estimates of the permeability for the deeper, coarser part of the aquifer. Average velocities for the Fairfax District based on maximum and minimum values for permeability and gradient were calculated to be 4.2×10^{-3} ft/day for silty units and 3.70 ft/day for sands and gravels. These compare well to velocities of 5×10^{-2} ft/day for silts and 1.3 ft/day for sands for the Fairfax District. (Fishel,1948)

Groundwater velocity will vary with depth because of changes in grain size and hydraulic conductivity. It will be greater in the sand than in the interbedded silts and clays. The velocity, direction, and gradient of the groundwater will also vary with the time of the year, as the Missouri River level changes. (GeoTrans,1985; Foreman,1979; SCS,1990; Katzman and Luce,1991). The position of recharge and discharge points will control local gradients, as mounding will occur where runoff is directed to drainage swales and man-made conduits. These factors may exert a temporary influence on gradients in the immediate vicinity of the site.

A generalized flow map has been constructed, based on flow directions constructed perpendicular to the potentiometric contours for the Fairfax District (Figure 5-1). The potentiometric contour positions are based on patterns observed during year-long monitoring of Missouri River/alluvial aquifer interaction at the Riverfront Landfill site, ten miles downriver from the Brinkerhoff Road facility (SCS, 1990; Katzman and Luce, 1991). Based on the Riverfront example, groundwater flow across the site is generally to the northeast, changing to a more easterly direction with increasing river underflow. This correlates well with the barely perceptible easterly gradient measured at the site. Potentiometric surface and groundwater flow across the site are presented in Figure 5-2.

5.3 Analytical Results

The ten on-site wells and piezometers were sampled and analyzed for volatile organic compounds (VOCs) (method 8240) and PCB compounds (method 8080/608) by Pace Laboratories.

Only two VOCs, trichloroethylene (TCE) and 1,2- dichloroethylene (1,2-DCE), and two PCBs, Aroclor 1242 and Aroclor 1260, were detected in some of the groundwater samples. Method detection limits varied from sample to sample, depending on the compound concentrations. The

analytical results and quality control data are presented in Appendix C. Table 5-3 summarizes the results by sampling point.

TCE was detected at levels ranging from 0.088 mg/L to 670 mg/L, and 1,2-DCE ranged from non-detect to 120 mg/L. All ten samples exceeded the Maximum Contaminant Level (MCL) for TCE 0.005 mg/L. Six of the ten samples exceeded the MCL for 1,2-DCE of 0.1 mg/L, and three samples were tested at method detection limits (0.25 to 20 mg/L) in excess of the MCL. Aroclor 1260 ranged from non-detect to 0.0230 mg/L, and one sample had measurable Aroclor 1242 at 0.0016 mg/L. The four samples with detectable PCB exceeded the MCL of 0.0005 mg/L.

No free phase chemical was found.

5.4 Groundwater Quality

Figure 5-3 is a logarithmic contour plot of TCE distribution. The highest TCE concentrations and the center of the plume are found near the northwest corner of the building, and the lowest TCE levels are located at the northeast corner of the property. The axis of the plume stretches in a general east-west direction, with TCE concentrations rapidly attenuating to approximately 1% of the highest values within 120 feet of the source. TCE was detected in both of the deep wells (MW-12B and MW-13B) at lower concentrations than the shallow wells (Table 5-3). The shape and extent of the TCE plume (Figures 5-3) is consistent with the groundwater gradient (Figure 5-2).

1,2-DCE was detected in five of the eight shallow wells and two of the deeper wells. 1,2-DCE was non-detect in the remaining wells, but the method detection limits were relatively high in at least two of the analyses (in excess of 2.5 and 20 mg/L). The method detection limit for the sample at MW-13B was 20 mg/L, and 1,2-DCE was detected at 24 mg/L in the blind duplicate sample from MW-13B (labeled MW-15A, Appendix C).

PCB was detected in the groundwater at two shallow wells (MW-13A and MW-11A) and one deep well (MW-13B). The highest PCB concentration (0.023 mg/L) was found at well MW-13A, outside the northwest corner of the building. Groundwater samples were not filtered prior to PCB analysis. Therefore, the low PCB concentrations may come from PCB which was sorbed on particulates in the water and not from dissolved aqueous phase.

Ratios of 1,2-DCE to TCE vary from a low of 0.07 to a high of 0.9 DCE/TCE. The ratios increase in a downgradient direction (compare Figure 5-2 with 5-4). This increase is expected, as DCE is formed by the degradation of TCE. The DCE concentration is expected to increase with time as the TCE concentration decreases. Decreased chemical concentrations in the deeper wells indicate little vertical migration of these analytes.

6.0 CHEMICALS OF CONCERN

The two chemicals of concern are polychlorinated biphenyls (PCB) and trichloroethylene (TCE). The only other chemical detected is 1,2-dichloroethylene (1,2-DCE), which is believed to be solely a "daughter" product from the reductive dehalogenation process of TCE. No historical data exists for the presence of 1,2-DCE at the site, and the dehalogenation process is well known. Figure 6-1 shows a schematic of the dehalogenation process.

This section discusses the chemical and physical properties of PCB and TCE.

6.1 <u>PCB</u>

Polychlorinated biphenyls are complex mixtures of chemicals composed of multi-ringed, chlorinated compounds. They are highly inert, persistent in the environment, and characterized by very low vapor pressures, low water solubilities, and high partition coefficients. They were used in the electrical transformer industry as insulator fluid additives for their excellent heat transport properties.

Aroclor 1260 (CAS# 11096-82-5) accounted for 87% of the PCB handled at the facility and is the chemical used for contaminant transport modeling. Aroclor 1260 is a mixture of biphenyls with varying degrees of chlorination (4 to 7 chlorine atoms per molecule) and is one of the heaviest, most strongly sorbed, and most environmentally persistent PCB. The properties of Aroclor 1260 as used for modeling are shown in Table 6-1.

PCB is labeled as a Class B2 carcinogen (probable human carcinogen) by the IRIS database (USEPA, 1994). This rating is based on studies which found direct evidence for formation of hepatocellular carcinomas in three strains of rats and two strains of mice. There is inadequate yet suggestive evidence of excess risk of liver cancer in humans by ingestion and inhalation or by dermal contact.

Although there is some evidence to suggest that PCB can degrade in the subsurface, it is assumed from a point of conservancy in the modeling that PCB does not degrade. Due to the presence of TCE in the contaminated soil, it is also assumed that the solubility is 1000 times greater and

sorption properties are 1000 times less than they would be otherwise. Further modeling assumptions are noted in Section 7.3, and modeling results are discussed in Section 7.4.

PCB has been found in groundwater only in areas where TCE is at or near its solubility limits (e.g. MW-13A). PCB cannot be transported in the groundwater unless TCE is present at concentrations near the solubility limit.

6.2 <u>TCE</u>

TCE (CAS# 79-01-6) is a common solvent with a clear, colorless, watery appearance and a chloroform-like odor. It is highly volatile and denser than water. It can biodegrade under reducing conditions, but it usually cannot be oxidized. Table 6-2 contains the physical/chemical properties as they were used for fate and transport modeling.

The carcinogenicity rating for TCE has been withdrawn by EPA, and a non-carcinogenic rating is pending. (USEPA,1994) It was formerly rated as a Class B2 carcinogen (probable human carcinogen), based on significant increases of liver tumors and malignant lymphomas in mice. There is inadequate evidence of human carcinogenicity. The Maximum Contaminant Level (MCL) for TCE (0.005 mg/L) is used for all risk/exposure scenarios.

The rate of biodegradation was used as a variable for calibrating the model to site conditions. Matching site conditions, the estimated biodegradation rate is less than 100 times that reported in the literature; therefore, the value is conservative.

7.0 CONTAMINANT TRANSPORT ANALYSIS

In order to assess the risk to human health and the environment from subsurface chemicals at the site, fate and transport modeling was used to help describe the chemical transport phenomenon. The most critical exposure pathway to subsurface contamination was found to be water ingestion. Modeling results indicate the TCE groundwater contamination will move eastward, towards industrial wells located about 0.8 miles from the site.

Even under the many conservative assumptions and supposed conditions for contaminant transport, groundwater contamination is shown to not impact human health or the environment. The forward edge of the modeled TCE plume, defined by the 0.005 mg/L line, will not move beyond 150 m (500 ft) from the source before completely dissipating. Both modeling and field data indicate that PCB will not migrate beyond the immediate source area.

The section discusses and evaluates: 1) the different pathways of exposure; 2) the conceptual and computer models used to evaluate chemical fate and transport; 3) the results of the computer modeling; 4) the elements of conservatism inherent in the modeling; and 5) the impact of the 1993 summer floods on the site groundwater quality.

7.1 Exposure Pathway Analysis

Exposure to the chemicals remaining in the subsurface may take place only through excavation of the soil or through pumping of the groundwater. Several exposure routes are discussed:

- Inhalation
- Water Ingestion
- Soil Ingestion
- Dermal Contact
- Particulate Inhalation

Application of specific site conditions to these exposure routes is discussed in Section 9.0 (Risk Characterization).

7.1.1 Inhalation of Vapors and Particulates

The contamination from TCE and PCB at the site is limited to strata far beneath the surface. PCB contaminated soil left unexcavated lies roughly 20 feet below grade. The TCE resides mostly in the groundwater and saturated strata. The source of contamination has been excavated from the pit area, leaving no surface or near surface source for exposure.

The only receptor by this pathway might be a worker who is digging or drilling on the property into the affected media. At that depth TCE could be released, but only in limited quantities that could cause an odor nuisance or minor health risk. While not volatile, PCBs can adhere to particles which may be inhaled.

Other potential risks to site personnel from inhalation are emissions from remediation technologies such as groundwater air stripping or soil vapor extraction. Off-gases from such technologies are treated to the proper health based level, based on the chemical. Aside from remediation activities, no vapors should escape as long as the subsurface remains undisturbed to the water table. Left undisturbed, there is no pathway for vapors from 20 ft below grade to the surface. The exposure potential is further limited by the ground covering at the site: concrete flooring in the building and asphalt paving in the parking lot and loading areas.

7.1.2 Soil Ingestion

As previously described, residual chemical present in the soil lies about 20 ft below the surface, underneath the building. The receptor would be a person who would ingest the contaminated soil. The building would need to be demolished, and the entire area would need to be excavated in order for the soil to be accessible. Typical building construction in the Fairfax District extends less than 10 feet down. The current building footer only extends about 6 feet below the surface. Thus, excavation to that depth in the immediate vicinity of the building is unlikely.

This scenario is improbable, and any exposure from soil ingestion is based on an almost impossible exposure route.

7.1.3 Dermal Contact

Similar to inhalation exposure, dermal contact with affected media can occur only in instances of drilling, excavation, or a similar action. Skin irritation can result from exposure to PCB and TCE contaminated soil. No other risks are involved with dermal exposure, and protective clothing (e.g., work gloves) is adequate prevention. MSDS sheets for PCB fluids and TCE are included as Appendix D.

7.1.4 Water Ingestion

While soil ingestion is unlikely, water contamination could migrate far offsite to potential receptors - in this case, drinkers of municipal or private well water. No drinking water wells are located in the Fairfax District; however, there are several water wells in the area of the General Motors plant, approximately 0.8 miles away (Figure 2-1).

PCB is very persistent in the soil and almost insoluble in water. TCE has a moderate solubility (Table 6-2) and is believed to have been transported in low concentrations off site. Contaminant fate and transport modeling is a very helpful technique for predicting the level and extent of contamination beyond the limited site data. This scenario and the risk to water drinking receptors are discussed Sections 7.2 and 9.0.

7.2 Groundwater Pathway Analysis

PCB is transported in the subsurface only under unusual conditions, as they sorb strongly onto the soil particles (see Section 6.0 for more detail). PCB will move only when:

- TCE is present in high concentrations, and PCB is coeluted with migrating TCE, or
- Small grained soil particles with sorbed PCB are transported.

Thus, the transport of PCB and TCE are inextricably linked. This solvency mechanism is what transported PCB to such a depth from the inside of the building.

Risk due to TCE and PCB migration is determined by using fate and transport analysis and examining the migration pathway of the groundwater as it relates to the Missouri River and

groundwater wells. A piezometric study was conducted of the site groundwater. The gradient over the site was found to be very shallow (0.076 %) to the east. This head difference (0.1 ft) is within the overall measurement error. The fact that it extends toward the east, as indicated in Figure 5-2, correlates well with the groundwater flow for the Fairfax District, shown in Figure 5-1. The flowlines indicated in Figure 5-1 are constructed from perpendiculars to the piezometric contours for the area.

No drinking water wells are located within the flowfield. The nearest wells, used only for industrial purposes, are located at the General Motors plant. These wells are about 0.8 miles away from the site and are closely aligned with the groundwater flowlines from the site area. These wells are a possible receptor for any chemicals that may travel that far, but the water is not used for drinking.

While the wells are the most likely receptors based on data obtained from the site investigation and the Fairfax District, the possibility of contamination to the Missouri River must also be considered. The most direct path to the Missouri River is due north from the site (0.7 miles); however, that path goes against groundwater flowlines. The more realistic path to the river would be through the well fields at the General Motors plant; therefore, the wells at General Motors are considered the most likely places for reception.

7.3 Fate and Transport Analysis

Fate and transport of PCB and TCE at the site were estimated using the EPA approved computer package RISKPRO. RISKPRO can be used to predict the environmental risks and effects due to the exposure from pollutants in the air, soil, and water. Specifically, the groundwater transport was modeled using the compartment model AT123D (Analytical Transient 1-2-3 Dimensional Model). AT123D predicts the lateral and vertical spread of a chemical plume through the groundwater (saturated zone) and estimates the chemical concentration within the groundwater at positions on a user-specified three-dimensional grid. It can handle constant as well as time-varying chemical releases to the groundwater from a single point, area, line or volume source. Chemical releases may be of an instantaneous, continuous, or finite duration. There is no mechanism in the model to account for free-phase ganglia or residual source material. A detailed technical explanation of AT123D is given by Yeh (1981).

In order to be manageable for mathematical modeling, the site hydrogeologic condition was simplified to simulate contaminant migration. The source was assumed to be the "footprint" of the pit area on the top of the aquifer. The aquifer was assumed to be infinitely wide and 80 ft deep. Only single component contamination can be modeled by AT123D; therefore, the approach was to calibrate the model using site data for TCE and then apply that model several years into the future for both PCB and TCE.

The chemicals were modeled as if they were released continuously from 1983 until the present, the assumption being that the pit was acting as the only source of contamination and as of August 1994, the source will have been removed. Any assumptions were made to be conservative using best engineering judgment and experience.

Pertinent physical/chemical data for TCE and PCB are presented in Section 6.0. Properties for TCE were inserted directly into the model. As is shown by the high adsorbance and low solubility of PCB, it greatly prefers the solid phase and resists aqueous solution. In order to model contaminant transport of PCB in the groundwater, it was assumed that the TCE in the groundwater caused a co-solvency effect. At high concentrations of TCE, PCB could perhaps enter the groundwater at concentrations higher than normal and be transported. To be conservative, no degradation of PCB was assumed to occur.

7.4 Model Results

Results from AT123D showing chemical distribution are superimposed on either a site map or a local map of the Fairfax District to relate the plume concentrations and distances traveled with time. Plots of actual model outputs are available as Appendix E.

7.4.1 TCE Model Results

Figure 7-1 shows the current TCE groundwater plume, modeled as closely to site conditions as possible. Year 11 indicates the current year, 1994, and all yearly references are to made to 1983 being Year 1. The plume is shown to extend beyond the facility fenceline, elongating to the east with the groundwater flow. The scale is logarithmic; contours are plotted on orders of magnitude. At the 0.005 mg/L level the plume extends about 115 meters to the east. The bulk of

the chemical is in a small area less than 50 meters from the source. Figure 7-2 shows the same plume as it relates to the local area of the Fairfax District.

As shown in Figure 7-2, TCE contamination, at the 0.005 mg/L level, extends about 35 meters west from the source. The source is located very near the west fenceline, and no wells were located west of the source. Because the groundwater flow is in the easterly direction, and advection/dispersion is the main transport mechanism, the model shows most contamination travels to the east. Figure 5-3 may be used for comparison of model results to actual field data.

The phenomenon of chemical movement to the east is more clearly pronounced in Figure 7-3 (Year 26, 2009), which shows the center of the plume will move approximately 100 meters downgradient from the source area in the next 15 years. This prediction rests on primarily on the assumption that the source of contamination was removed. Due to the degradation of TCE, as already indicated at the site, the 0.005 mg/L level has migrated only to about 150 meters from the source, and the highest concentration at the center is about 0.01 mg/L. By Year 31 (Figure 7-4) the center of the plume has moved 125 meters east of the source, with the highest concentration at about 0.005 mg/L. With time, the plume continues to travel with the groundwater in the easterly direction, though only at residual concentrations (below 0.005 mg/L).

Model results show that by the present time (Year 11), TCE has reached the bottom of the aquifer, largely due to advective/dispersive forces. Appendix E contains the model results, plotted as length along the plume axis (X) against depth (Z). As was found in the site investigation, concentrations decrease with depth near the source (MW-12A,B and MW-13A,B, Table 5-3). Further from the source, the concentration profile flattens vertically, indicating TCE becomes evenly distributed over the depth of the aquifer. By Year 26, TCE concentrations at depth are predicted to be less than the 0.005 mg/L, with higher concentrations at the surface.

7.4.2 PCB Model Results

Model results indicate that PCB will remain highly localized. Figure 7-5 shows the model results for Year 11 (present) plotted onto a map of the site. Modeled PCB concentrations near the source are much higher than the field data is order to simulate PCB transport. Wells further away from the source indicate no presence of PCB, yet the model predicts concentrations to be on the order of part per billion. By Year 26 (Figure 7-6), PCB is predicted to have dissipated only slightly, with the plume gradually moving to the east. Figure 7-7 (Year 71) shows that PCB

concentrations remain high (ppm), and the plume retains its size and position; it moves and dissipates very slowly. The plume is much more symmetrical from east to west than it was for TCE, though the plume center is about 35 to 40 meters east of the source. The model results are essentially an extreme worst case scenario, since it was assumed that TCE was present in high quantities everywhere to facilitate PCB transport.

In assuming that the sorption for PCB is lower and the solubility is higher by several orders of magnitude, it was also assumed that TCE would be present in high concentrations to facilitate transport. This is perhaps the case in and around the source, but not at distances greater than 15 m from the source. Also, to show any transport, PCB loadings had to be raised to very high levels. PCB mass loadings to the aquifer were raised to 1% that of TCE (Table 7-1). The effect of this high loading is found at the center of the modeled plumes, in the PCB concentrations which are several orders of magnitude higher than observed in the field.

Even with the conservative modeling assumptions, PCB is not predicted to reach the bottom of the aquifer. The plume is shown to descend about halfway into the aquifer before dissipating below calculable levels.

7.5 Modeling Conservatism

Several assumptions were made to produce models of TCE and PCB contamination in the groundwater. Whenever possible, conservative values were used to provide assurance of credibility.

For modeling TCE, physical properties from the literature were used whenever possible. Model input parameters are indicated in Table 7-1. Input parameter values are displayed as either given values (from a text or other source), variables, or assumptions. In all cases, efforts were made to use conservative estimates for input variables. The main parameters which were varied to simulate site conditions were contaminant loading (rate of chemical introduction to the subsurface) and average hydraulic conductivity. Less important (fine tuning) variables were longitudinal, transverse, and vertical dispersivities, and the chemical degradation rate.

Several modeling parameters and situations were assumed so as to present conservative estimates of contaminant transport. The hydraulic conductivity was assumed to be $1x10^{-2}$ cm/sec for the entire depth of the aquifer, but it is probably considerably less in the silty clay strata where most

of the contamination resides. The effective porosity, i.e. the connected pore space, was assumed to be 28%, though it is probably much lower in the fine-grained top strata. Soil organic content, the chief sorptive material in soils, was assumed to be 0.1%, even though evidence suggests it is much higher (boring logs, Appendix B).

To simulate site conditions, the release rate for TCE was assumed to be 0.015 kg/hr for 137 months (1983 to 1994). This translates to 1480 kg, or 2100 L, of TCE released over 11 years. In reality, nearly all the TCE was released during a three year period (1983-1986) early in the history of the transformer reclamation facility (Section 2.0). The biodegradation rate for TCE and the longitudinal, transverse, and vertical dispersivities were used to estimate the size and extent of the groundwater plume as indicated by the field investigation.

PCB release was assumed to be 1/100 that of TCE. This is undoubtedly a conservative figure, as the PCB concentration in the original transformer fluids was less than 1% (Appendix D). The actual ratio of PCB to TCE was probably on the order of part per million. The coefficient of retardation (K_D) for PCB was assumed to be 500 times smaller than the average literature value, on the basis that high concentrations of TCE produce a co-solvency effect and transport the PCB. This may be the case at or very near the source area (the "pit"), but is not the case anywhere else. Modeled concentrations of PCB are over 2 orders of magnitude higher than field data suggest, indicating a highly conservative scenario. It is likely that PCB is not in the groundwater at all, because the low concentrations found by GCMS were determined for unfiltered samples, which may have had PCB sorbed on suspended particulates.

7.6 Flood Analysis

In response to the 1993 Midwest floods and the situation described in Section 2.0, an analysis was made to determine the possible effects the flood might have had on contaminant transport. At the time, the source material was still in place, and excavation at the pit had just begun. Equilibrium relationships were used to predict the maximum amount of chemical that could have been transported into the aquifer from the source material. Based on the equations and calculations shown in Appendix F, approximately 5 kg of TCE and 1 g of PCB could have moved from the sorbed phase to the water soluble phase and been transported into the aquifer. This represents only a small fraction (0.3%) of the total chemical introduced to the groundwater and is insignificant when considering the overall contamination scenario.

8.0 CURRENT AND FUTURE LAND USE

Current land use at and near the Brinkerhoff Road site is zoned Heavy Industrial only. The entire Fairfax District is zoned as Heavy Industrial and is likely to remain so in the future. The Fairfax District encompasses the entire groundwater flow path from the site. There are no residential areas within the Fairfax District, and no groundwater from the Fairfax District is used for municipal purposes.

9.0 RISK CHARACTERIZATION

EPA guidance states that remediation to a 10⁻⁴ or 10⁻⁶ risk level is not necessary for groundwater that is not a potential source of drinking water. Nevertheless, because groundwater ingestion was found to be the only credible exposure pathway, the MCL for PCB (0.0005 mg/L), which falls between those risk levels, was used to present a conservative estimate of groundwater contamination. The MCL for TCE (0.005 mg/L) was used to define the TCE groundwater plume.

Despite highly conservative assumptions and scenarios for exposure, no chemical contamination was found to reach a receptor or otherwise impact anyone or anything now and into the future. TCE is predicted to naturally attenuate well before reaching any possible receptor, and PCB modeling predicts the chemical will not leave the immediate site area, even under the highly conservative transport scenario. No further remediation is necessary to reduce risk to human health or the environment.

This section assesses the potential risk to receptors by: selecting the appropriate risk levels and corresponding chemical concentrations; and considering all the information assembled in this report to form a basis for risk characterization.

9.1 Selection of Maximum Allowable Exposure Concentrations

In the proposed RCRA, Part 264, Subpart S regulation (USEPA, 1990B), EPA suggested a lifetime, on-site, residential exposure scenario for the purpose of calculating an action level below which: 1) no further action is necessary, and 2) the contamination level is considered safe. In deriving action levels for hazardous constituents in groundwater, a daily water intake rate of 2 liters per day over a 70 year lifetime exposure period is assumed.

Depending upon site-specific or remedy specific factors, the acceptable risk level may range from 10^4 to 10^6 for Class A and B carcinogens. PCB is considered a Class B carcinogen which is accumulated in the body over a lifetime to simulate a conservative exposure scenario. The carcinogenicity rating for TCE has been withdrawn, and a non-carcinogenic rating is pending. (USEPA,1994) (see Section 6.0 for further discussion on chemical carcinogenicity).

In the preamble of the proposed Subpart S regulation, EPA further stated that:

"Groundwater that is not a potential source of drinking water would not require remediation to a 10⁻⁴ to 10⁻⁶ level.Finally, contaminated soil at an industrial site might be cleaned up to be sufficiently protective for industrial use but not residential use, as long as there is reasonable certainty that the site would remain industrial."

In March 1991, EPA issued a new guidance (USEPA, 1991B) on standard default exposure factors for risk assessment. In the new guidance, the lifetime exposure is defined as 30 years for a single location. EPA also reduced the exposure time from 365 days/year to 350 days/year. For industrial exposure, EPA suggested a daily water intake rate of 1 liter/day, at 250 days/year, for 25 years as lifetime exposure.

Using the above guidance with risk equations developed by EPA (1991A), the Maximum Allowable Exposure Concentration (MAEC) for PCB was calculated for different risk levels. A spreadsheet detailing the calculations is found in Appendix G. Using an oral slope factor of 7.7 (mg/kg/day)-1 (USEPA,1994), the action levels for PCB by the water ingestion pathway are:

- 10-4 risk 0.00111 mg/L; and
- 10⁻⁶ risk 0.0000111 mg/L.

The Maximum Contaminant Level (MCL) for PCB is 0.0005 mg/L, which is between the 10⁻⁴ and 10⁻⁶ risk levels. Thus, the MCL presents a conservative MAEC for an industrial scenario, in which no groundwater is used for drinking, and no human habitation exists or is ever planned.

The MCL for TCE (0.005 mg/L) is used as the MAEC for water ingestion pathway scenarios.

9.2 Risk Characterization

Given the site conditions and no credible exposure scenario, risk from soil contamination cannot be determined (Section 7.1). The PCB contaminated soil is estimated to be about 37 yd³, located 20 ft below the ground surface, and covered by either concrete or asphalt. This soil is unable to be excavated without complete destruction of the site property (Section 2.3). There is no exposure route or mechanism for chemical transport through the 20 ft of clean soil. Thus, risk

from PCB contaminated soil, due to inhalation, ingestion, or dermal contact cannot be realistically assessed.

Risk from contaminated groundwater (PCB and TCE) can be considered by modeling the transport of contaminants from the site eastward to wells at the General Motors Plant and northward to the Missouri River, each of which are approximately 0.8 miles away. Figures 7-1 through 7-7 illustrate the potential impact to human receptors, as predicted by fate and transport modeling. No TCE or PCB contaminated groundwater is modeled to reach the wells or the Missouri River at concentrations greater than the MAECs.

The TCE groundwater plume, defined by the 0.005 mg/L MAEC, is presumed to have migrated from the source at the site to beyond the property boundary. Figures 7-1 through 7-4 show that in as little as 30 years from the present, the TCE will naturally attenuate below drinking water concentrations and will never reach a potential receptor. TCE contaminated groundwater is predicted to travel only about 150 meters, about one-tenth the distance to the wells, before dissipating below the 0.005 mg/L MAEC.

PCB has been found in some wells very close to the source, but its presence is believed to be due to either TCE coelution (the presumed method of vertical migration from the source) or faulty analytical procedures (samples were unfiltered). PCB contaminated groundwater, as modeled under a highly unlikely and conservative transport scenario (Section 7.5), is predicted to remain in the immediate site area. Figures 7-5 through 7-7 show model predictions indicating that even under highly conservative and unlikely conditions, PCB will not migrate beyond the site area. Site investigation results indicate PCB has not spread beyond the immediate source area (the "pit") (Section 5.4). There are no indications of PCB in any of the wells downgradient of the source.

One significant modeling assumption is that the source of contamination has been removed as of August, 1994. This has largely been achieved, with only an approximate 28 yd³ of the source material remaining unexcavated. Because the Fairfax District is an area which is zoned Heavy Industrial only, and is likely to remain so long into the future, the relatively small amount of groundwater contamination from the site presents an insignificant risk to human health or the environment.

There is no known sensitive environmental receptor in the Fairfax District.

In conclusion, the remaining subsurface contamination resulting from operations at the site presents no significant risk to human or environmental receptors. Removal of the source material is sufficient to preclude any further contamination of the alluvial aquifer, and no further remedial activity is warranted to reduce risk to workers, the populace or the environment.

10.0 CONCLUSIONS

- 1. The Binkerhoff Road site is situated in the Central Lowlands Physiographic Province, and the topography is dominated by the floodplains and bluffs of the Missouri River. The first bedrock (Pleasanton) is relatively impermeable and about 70 ft thick at the site. The alluvial soils in the immediate Missouri floodplain can be broken down into three regions:
 - Upper 40% of vertical profile silt, clay, and very fine sand;
 - Middle 40% medium sand with interlayers of silt and coarse sand; and
 - Lower 20% coarse sand and gravel.
- 2. Findings from the site investigation indicate that TCE has entered the groundwater and spread from the presumed source (the "pit" area) to the property boundaries, primarily in the eastward direction of groundwater flow. TCE concentrations range from 670 mg/L near the source to 0.088 mg/L near the east fenceline. TCE is being transformed to 1,2-DCE, which was found at most wells on the property. No free phase chemical was found.
- 3. PCB was detected at very low concentrations (≤ 0.023 mg/L) in the groundwater near the source; however, the levels may be due to sorbed PCB on soil particles. Groundwater samples were not filtered prior to analysis.
- 4. Remedial activities conducted at the site have removed all PCB contaminated surface material and over 90% of the PCB and TCE contaminated "pit" area, the source of the groundwater contamination. Complete destruction of the site and neighboring properties would be necessary to completely remove all the PCB contaminated soil.
- 5. The only credible exposure pathway to subsurface contamination is by water ingestion.
- 6. Results from fate and transport modeling indicate the TCE plume will move towards industrial wells located about 0.8 miles from the site. The forward edge of the TCE plume, as indicated by the 0.005 mg/L MAEC, will not move more than 150 m (500 ft) from the source. Therefore, the plume will not impact the downstream wells or any other point of reception.

- 7. Even under the highly conservative conditions for PCB transport (e.g., TCE coelution and high mass loadings), modeling results and field data indicate that PCB will not migrate toward any receptor.
- 8. Current land use at and near the site is zoned Heavy Industrial only. The entire Fairfax District is zoned as Heavy Industrial and is likely to remain so in the future. The Fairfax District encompasses the entire groundwater flow path from the site. There are no residential areas within the Fairfax District, and no groundwater from the Fairfax District is used for municipal purposes.
- 9. Based upon the results of this risk assessment and the fate and transport modeling, no further remedial action is warranted for the soil and groundwater in and around the Brinkerhoff Road facility.

REFERENCES

- Barbee, G. C. (1994) <u>Fate of Chlorinated Aliphatic Hydrocarbons in the Vadose Zone and Ground Water</u>. Ground Water Monitoring and Remediation. v. 14, pp. 129-140.
- City of Kansas City, Kansas. (1986) Contract Documents for Well Replacement, Fairfax.

 <u>Drainage District. Kansas City, KS.</u> Manuscript accompanied by boring logs and geotechnical grain size analyses.
- Clement, E. B., M. M. Katzman and A. C. Moll (1994) <u>Assessment and Closure of the Riverfront Landfill. Kansas City. Missouri.</u> Air and Waste Management Association, 87th Annual Meeting, Cincinnati. In press.
- Crabtree, J. and P. Malone, (1984). <u>Hydreogeologic Characterization of Conservation Chemical Company Site. Kansas City. Missouri</u>. Report prepared for USEPA Region VII, Kansas City, Missouri, under IAG No. DW930105-01-0. Geotechnical Laboratory, U. S. Army Engineers Waterways Experiment Station, Vicksburg, MS.
- Dort, Wakefield, Jr. (1987) "Salient Aspects of the Terminal Zone of Continental Glaciation in Kansas". pp. 54-66 in <u>Ouaternary Environments in Kansas</u>, W. C. Johnson, editor. Kansas Geological Survey, Guidebook Series 5. Lawrence, KS. 208 p.
- Dufford, A. E. (1958) Quaternary Geology and Groundwater Resources of Kansas River between Bonner Springs and Lawrence Kansas. State Geological Survey of Kansas, Bulletin 130, Lawrence, KS 96p.
- Emmett, L. F. and H. G. Jeffery (1969). <u>Reconnaissance of the Groundwater Resources of the Missouri River Alluvium Between Kansas City, Missouri and the Iowa Border.</u> USGS Hydrologic Investigations Atlas HA-336. Washington, D.C.
- Emmett, L. F. and H. G. Jeffery (1970). <u>Reconnaissance of the Groundwater Resources of the Missouri River Alluvium Between Miami and Kansas City, Missouri.</u> USGS Hydrologic Investigations Atlas HA-344. Washington, D.C.
- Fader, S. W. (1974) <u>Ground Water in the Kansas River Valley, Junction City to Kansas City.</u>
 <u>Kansas</u>. Kansas Geological Survey Bull. 206, Part 2, Lawrence, KS. 12p.
- Fishel, V. C. (1948) <u>Groundwater Resources of the Kansas City, Kansas, Area.</u> Kansas Geological Survey, Bull 71. 109p.
- Fishel, V. C., J. K. Searcy and F. H. Rainwater (1953). <u>Water Resources of the Kansas City Area, Missouri and Kansas.</u> USGS Circ. 273, Washington, D.C. 53 p.

- Foreman, T. L. (1979) <u>Determination of Hydrogeologic Properties of Missouri River Alluvium</u>
 <u>Using Numerical Modeling Techniques</u>. M.A. Thesis, University of Missouri at Columbia. 124p.
- Freeze, R. A., and J. A. Cherry (1979). <u>Groundwater</u>. Englewood Cliffs, N. J. Prentice Hall, Inc. 604 p.
- Gann, E. E., E. J. Harvey, J. H. Barks, D. L. Fuller and D. E. Miller (1974). <u>Water Resources of West-Central Missouri</u>. USGS Hydrologic Investigations Atlas HA-49. Washington, D.C. 4 sheets.
- GeoTrans, Inc. (1985). <u>Hydrogeologic Analysis of the Conservation Chemical Company Site.</u>
 <u>Kansas City, Missouri</u>. Report to USEPA, Region VII, Kansas City, Missouri under contract to PRC Engineering. Herndon, VA. 19p.
- Hasan, S. E., R. L. Moberly, and J. A. Caoile (1988). "Geology of Greater Kansas City, Missouri and Kansas, United States of America," <u>Bull. Assoc. Engineering Geologists</u>, V. 25, No. 3, pp. 277-342.
- Heath, R. C. (1983) <u>Basic Ground-Water Hydrology</u>. USGS, Professional Paper 220. Reprinted, 1991 Washington, D.C. 84 p.
- Heckel, P. H., L. L. Brady, W. J. Ebaks, Jr. and R. K. Pabian 1979) <u>Field Guide to Pennsylvanian Cyclic Deposits in Kansas and Nebraska</u>, pp. 4-60 in <u>Pennsylvanian Cyclic Platform Deposits of Kansas and Nebraska</u>. Kansas Geological Survey, Guidebook Series 4, Lawrence, KS. 79 p.
- Jewett, J. M., H. G. O'Connor and W. J. Seevers (1965) <u>Hydrogeology of the Lower Kansas River Valley</u>, Geol. Soc. Amer. 1965 Annual Meeting Field Conference Guidebook, Kansas City, MO. 45p.
- Katzman, M. M. and R. L. Luce (1991) <u>Missouri River-Alluvial Aquifer Groundwater Interaction, Kansas City, Missouri.</u> Abst. EOS, Transactions American Geophysical Union, V. 72, p. 185.
- Layne GeoSiences, Inc. (1990) <u>Trans-Missouri River Tunneling Project Geohydrology Report.</u>
 Unpublished report. 2 vols. Kansas City, KS.
- Layne-Western Company, Inc. (1984). <u>Project Review, Fairfax Industrial Development, Kansas City, Kansas.</u> Report prepared for Kansas City, Kansas Port Authority. 27 p. Kansas City, KS. 27p.
- MacFarlane, P. A. and L. R. Hathaway (1987) <u>The Hydrogeology and Chemical Quality of</u>
 Groundwaters from the Lower Paleozoic Aquifers in the Tri-State Region of Kansas.

- <u>Missouri and Oklahoma</u>. Kansas Geological Survey Ground Water Series No. 9. Lawrence, KS 37p.
- Montgomery, J.H. and L.M. Welkom, (1990) <u>Groundwater Chemicals Desk Reference</u>, Lewis Publishers, Inc., Chelsea, Michigan.
- Netzler, B. W. (1982) Map of Total Dissolved Solids Concentrations in Groundwater from the Lower Pennsylvanian (Atokan Series and Cherokee Group) in Missouri. Open File Map OFM-82-60-WR, Mo. Dept. Nat. Res., Div. Geology & Land Surv., Rolla, MO. 1 sheet.
- Netzler, B. W. (1982) Map of Total Dissolved Solids Concentrations in Groundwater from the Mississippian Aquifers in Missouri. Open File Map OFM-82-61-WR, Mo. Dept. Nat. Res., Div. Geology & Land Surv., Rolla, MO. 1 sheet.
- Neyer, Tiseo & Hindo, Ltd. (1985). <u>Hydrogeologic Impact Study of Proposed Groundwater</u>

 <u>Pumpage at the New General Motors CPC Fairfax Plant</u>. Report prepared for General

 Motors Argonaut Architecture Engineering and Construction. Farmington Hills, MI. 41p.
- O'Connor, H. G. (1971) <u>Geology and Ground-Water Resources of Johnson County, Northeastern Kansas.</u> Kansas Geological Survey, Bulletin 203. Lawrence, KS 68 p.
- O'Connor, H. G. (1973) <u>Geohydrology for Urban Planning in Johnson County, Northeastern Kansas</u>. Kansas Geological Survey, Open File Report 73-9. Lawrence, KS 41 p.
- O'Connor, H. G. and L. W. Fowler (1963). <u>Pleistocene Geology of a Part of the Kansas City</u>
 <u>Area</u>. Trans. Kansas Academy of Science. v. 66, pp. 622-631.
- Palmer, J. R. (1981) <u>Isopachous Map of the Mississippian System in Missouri</u>. Open File Map OFM-81-36-G1. Mo. Dept. Nat. Res., Div. Geology & Land Surv., Rolla, MO. 1 sheet.
- Palmer, J. R. (1981) <u>Isopachous Map of the Pennsylvanian System in Missouri</u>. Open File Map OFM-81-38-G1. Mo. Dept. Nat. Res., Div. Geology & Land Surv., Rolla, MO. 1 sheet.
- Parizek, E. J. (1975). <u>The Geologic Setting of Greater Kansas City, Missouri-Kansas.</u> pp 9-23 in Stauffer, T. and J. D. Vineyard, editors. Proceedings of the Symposium on the Development and Utilization of Underground Space. National Science Foundation, Washington, D.C.
- Plinsky, R. O., J. L. Zimmerman, H. O. Dickey, G. N. Jorgensen, R. W. Fenwick and W. E. Roth. (1979). Soil Survey of Johnson County, Kansas. U.S. Department of Agriculture, Soil Conservation Service, Washington, D.C. 93 p.
- Reed, T. B. and R. D. Burnett (1985) <u>Compilation and Analyses of Aquifer Performance Tests in Eastern Kansas.</u> U.S.G.S. Open File Report 85-200. Lawrence, KS. 125p.

- SCS Engineers (1990). <u>Groundwater Investigation, Riverfront Site, Kansas City, Missouri.</u>
 Report to U.S. EPA, Region VII, under contract to City of Kansas City, Missouri Health Department. Kansas City, MO.
- Simms, J. J. (1967). A Study of the Bedrock Valleys of the Kansas and Missouri Rivers in the Vicinity of Kansas City. Unpublished MS Thesis. University of Kansas, Lawrence KS. 105 p.
- Upchurch, S. B. (1994). <u>Hydrochemical Facies Analysis for Site Contamination Evaluation and characterization.</u> Unpublished manuscript, 1994 ERM Excellence Awards Technology Exchange, ERM, Inc., Exton, PA. 9p.
- U.S. Army Corps of Engineers (1979). <u>Kansas City Urban Study</u>. <u>Appendix 1, Background Information</u>. USACOE Kansas City District, Kansas City, MO.
- U.S. Army Corps of Engineers (1953). <u>Preliminary Examination for Flood Control for Proposed Missouri River Cut-Off, Vicinity of Kansas Cities, Missouri and Kansas.</u> USACOE Kansas City District, Kansas City, MO.
- U.S. Army Corps of Engineers (1979). Operation and Maintenance Manual, Kansas City Flood
 Control Project, Fairfax-Jersey Creek Unit. Vol. 1. USACOE Kansas City District,
 Kansas City, MO.
- USEPA, (1990A) <u>Guidance on Remedial Actions for Superfund Sites with PCB Contamination</u>, EPA/540/G-90/007. (PB91-921206)
- USEPA, (1990B) "Corrective Action for Solid Waste Management Units (SWMUs) at Hazardous Waste Management Facilities," Federal Register, 55, FR 30798-30884.
- USEPA (1991A) <u>Risk Assessment Guidance for Superfund: Volume 1 Human Health</u>
 <u>Evaluation Manual (Part B, Development of Risk-Based Preliminary Remediation Goals)</u>.
 EPA/540/1-89/002, Publ.# 9285.7-01B.
- USEPA, (1991B) <u>Human Health Evaluation Manual</u>, <u>Supplemental Guidance</u>: <u>Standard Default Exposure Factors</u>, OSWER Directive 9285.6-03, March 25, 1991.
- USEPA, (1994) Integrated Risk Information System (IRIS database). Update: April, 1994. PB-Number: PB91-591330.
- Winslow, J. D. and C. E. Nuzman (1965) "Electrical Analog of the Alluvial Aquifer in Kansas Valley near Topeka, Kansas," pp. 30-38 in Hydrogeology of the Lower Kansas River Valley, (Jewett, J. M. et. al.), Geol. Soc. Amer. 1965 Annual Meeting Field Conference Guidebook, Kansas City, MO. 45p.

- Woodward-Clyde Consultants (1990a) <u>Trans-Missouri River Tunnel Geotechnical Data Report</u>, unpublished report, 2 volumes. Overland Park, KS. 50p.
- Woodward-Clyde Consultants (1990b) <u>Trans-Missouri River Tunnel Geotechnical Interpretive</u> <u>Report</u>, unpublished report. Overland Park, KS. 29p.
- Yeh, G.T. (1981) AT123D: Analytical Transient One-, Two-, Three-Dimensional Simulation of Waste Transport in the Aquifer System. Oak Ridge National Laboratory, Publ. # ORNL-5602.
- Zatezalo, M. P. (1977). <u>Hydrogeologic Evaluation of Pollutant Dispersion From Municipal Sewerage Lagoons Located on a Floodplain</u>. Unpublished MA Thesis. University of Missouri at Columbia. 79 p.
- Zaveskey, L. D. and W. C. Boatwright (1977) <u>Soil Survey or Leavenworth and Wyandotte</u> <u>Counties, Kansas</u>. USDA, Soil Conservation Service, Washington, D.C. 80 p.
- Zeller, D. E. (1968) <u>The Stratigraphic Succession in Kansas</u>. Geological Survey of Kansas, Bull 189. Lawrence, KS. 81 p.

TABLES

Table 2-1 Soil Sampling Results (mg/kg)

Pace Lab (EPA) At 13' Depth Below Footer

UNISON Lab (Split Sample)

Cell I	TCE	<u>PCB</u>	PCB
(1) (2) (3) X S	ND 24 13 12.4 9.8 22.1	ND 49.3 <1.0 16.8 23.0 39.8	ND 85.8 5 28.8 40.3 69.1
Cell II	<u>ICE</u>	<u>PCB</u>	<u>PCB</u>
(1) (2) (3) X S	ND ND .7 0.3 0.3 0.6	63.5 0.1 50.3 37.9 27.3 65.3	73.0 BDL 92.3 55.1 39.7 94.8
Cell III	TCE	PCB	<u>PCB</u>
(1) (2) (3) X S	1.4 16 <u>ND</u> 5.8 7.2 13.0	13 9.8 <u>55.3</u> 26.0 20.7 46.8	57.7 4.9 <u>68.6</u> 43.7 27.8 71.5

REFERENCE INFORMATION Sample Orientation:

(Typical Call)



Clean Up Criteria:

TCE < 60 ppm UCL(95%) PCB < 10 ppm UCL(95%)

Table 3-1: Regional Pennsylvanian Stratigraphic Column

Group	Formation
	Topeka Limestone
	Calhoun Shale
Shawnee	Deer Creek Limestone
	Tecumseh Shale
	Lecompton Limestone
	Kanwaka Shale
	Oread Limestone
Douglas	Lawrence Shale
	Stranger Sandstone
Pedee	Weston Shale
	Stanton Limestone
Lansing	Vilas Shale
	Plattsburg Limestone
	Bonner Springs Sandstone
	Wyandotte Limestone
	Lane Shale
	Iola Limestone
	Chanute Shale
Kansas City	Drum Limestone
-	Cherryvale Shale
	Dennis Limestone
	Galesburg Shale
	Swope Limestone
	Ladore Shale
	Hertha Limestone
	Tacket Shale
Pleasanton	Checkerboard Limestone
	Seminole Shale-Sandstone
	Holdenville Shale
·	Lenapah Limestone
	Nowata Shale
Marmaton	Altamont Limestone
	Bandera Shale
	Pawnee Limestone
	Labette Shale
	Fort Scott Limestone
Cherokee	Cabaniss Shale-Sandstone
	Krebs Sandstone

modified after Hasan et al. (1988)

Table 4-1: Regional Bedrock Aquifer Quality And Yields

Bedrock Unit	Water Quality *	Yields (gpm)
	Fresh to slightly saline at	
Douglas, Lansing and	depths <100ft, locally	0-50, commonly
Kansas City Groups	<250ft. Moderate to very	less than 10
	saline >250ft depth	
Pleasanton Group	Very saline	0-20
Marmaton Group	Very saline	0-150
Cherokee Group	Very saline or briny	0-200
Mississippian rocks	Very saline	50-500
Hunton Group, Viola Ls.	Very saline or briny	0-100
Simpson Group	Very saline or briny	0-300
Arbuckle Group, Lamotte ss	Very saline or briny	200-1,000

modified after O'Connor (1971)

* Key:

Fresh = < 1,000ppm dissolved solids Slightly saline = 1,000 to 3,000 ppm dissolved solids Moderately saline = 3,000 to 10,000 ppm dissolved solids Very saline = 10,000 to 35,000 ppm dissolved solids Briny = > 35,000 ppm dissolved solids

Table 4-2: Generalized/Average Regional Aquifer Characteristics

Aquifer Type	Soil/Glacial	Regional Bedrock	Alluvial
Yield (gpm)	0.3 - 3.0	0.02 - 20.0	200 - 2,500
Discharge (ft³/day)	1,900 - 251,000	700 - 76,000	400 - 233,000
Transmissivity (ft²/day)	100 - 84,000	4 - 7,500	400 - >400,000
Storage Coefficient (dim)	0.15 - 3.0x10 ⁻⁴	0.15 - 8x10 ⁻⁴	0.27 - 2.8x10 ⁻⁴
Specific Capacity (gal/min/ft)	NA	0.45 - 111.0	14.0 -116.0

Table 4-3: Generalized Site/Near-Site Alluvial Aquifer Characteristics

Characteristic	Values
Yield (gpm)	80 - 1,580, Avg: 980
Transmissivity (gpd/ft)	400 - 825,000
Storage Coefficient (dim.)	0.27 - 2.8x10 ⁻⁴
Specific Capacity (gal/min/ft)	13.3 - 375.0, Avg: 60
Permeability (gal/day/ft²)	192 - 7,313
Hydraulic Conductivity (cm/sec)	9.0x10 ⁻³ - 3.4x10 ⁻¹
Recharge Rate (in)	5 - 15
Hydraulic Gradient (ft/mile)	4 - 5
Groundwater Velocity (ft/day)	4.2x10 ⁻³ (ML) - 3.70 (SP/GP)
Alluvium Thickness (ft)	65 - 145
Aquifer Thickness (sat. ft)	43 - 123
Bedrock Elevation (msl)	600 - 680
Surface Elevation (msl)	736 - 750

Table 4-4: Site Alluvial Aquifer Characteristics

Characteristic	Value	
Surface Elevation (ft-msl)	745	
Bedrock Elevation (ft-msl)	645	
Alluvium Thickness (ft)	Upper (ML, SM) 60 Lower (SP, GP) 40	
Aquifer Thickness (sat. ft)	Upper (ML,SM) 15 (23%) Lower (SP,GP) 65 (77%)	
Hydraulic Gradient (ft/mile)	4	
Yield (gpm)	1,000	
Transmissivity (gpd/ft)	400,000	
Storage Coefficient (dim.)	0.15	
Specific Capacity (gal/min/ft)	60	
Permeability (gal/day/ft²)	Average: 5,000 Upper (ML,SM): 600 Lower (SP,GP): 6,015	
Hydraulic Conductivity (ft/sec)	Average: 7.68x10 ⁻³ Upper (ML,SM): 9.21x10 ⁻⁴ Lower (SP,GP): 9.24x10 ⁻³	
Groundwater Velocity (ft/day)	2.02 (SP)	

Table 4-5: Characteristics Of Floodplain Soils*

U.S.C.	Alluvium Name	Hydraulic Conductivity (K, cm/sec)
GW/SP	Well graded gravels or gravel-sand mixtures, little or no fines	10-2
SP	Poorly graded sands or gravelly sands, little or no fines	10 ⁻³
SM	Silty sands, sand-silt mixtures	10 ⁻³ to 10 ⁻⁶
SC	Clayey sands, sand-clay mixtures	10 ⁻⁶ to 10 ⁻⁸
ML	Inorganic silts and very fine sands, silty or clayey fine sands or clayey silts	10 ⁻³ to 10 ⁻⁶
CL	Low to medium plastic inorganic clays, gravelly clays, sandy clays, silty clays	10 ⁻⁶ to 10 ⁻⁸
CH	Highly plastic inorganic clays	10 ⁻⁶ to 10 ⁻⁸

^{*} after Crabtree and Malone (1984)

Table 5-1: Well/Piezometer Data

Boring/Well/	Surface			Screen		
Piezometer	Elevation	Boring	Depth	Elev	Length	Surface
Number	(msl)	Depth (ft)	(ft)	(msl)	(ft)	Finish
S-1	745	60	N/A			None-sealed
PZ-1	744	32	32	712	20	flush mount
PZ-2	745	32	32	713	20	Steel casing
PZ-3	748	33	32	716	10	flush mount
PZ-4	744	33	32	712	10	flush mount
MW-11A	740	28	25	715	10	riser pipe
MW-12A	744	33	30	714	10	riser pipe
MW-12B	744	54	49	695	10	riser pipe
MW-13A	745	33	30	715	10	flush mount
MW-13B	745	54	49	696	10	flush mount
MW-14A	745	32	30	715	10	flush mount

Table 5-2: Groundwater Elevations

Well/Piezometer	Elevation* (ft-msl)
PZ-1	727.50
PZ-2	727.64
PZ-3	727.63
PZ-4	727.54
MW-11A	727.64
MW-12A	727.59
MW-12B	727.60
MW-13A	727.63
MW-13B	727.63
MW-14A	727.61

^{*} Measurement date - May 19, 1994

Table 5-3: Summary Of Analytical Laboratory Results

Well Number	Analyte	Result (mg/L)	MDL (mg/L)
PZ-1	Trichloroethylene	2.4	0.1
	1,2-Dichloroethylene	2.1	0.1
PZ-2	Trichloroethylene	25.0	2.5
	1,2-Dichloroethylene	ND	2.5
PZ-3	Trichloroethylene	4,100	0.25
	1,2-Dichloroethylene	0.29	0.25
PZ-4	Trichloroethylene	0.088	0.005
	1,2-Dichloroethylene	0.013	0.005
MW-11A	Trichloroethylene	360.0	10.0
	1,2-Dichloroethylene	120.0	10.0
	Aroclor 1242	0.0016	0.001
	Aroclor 1260	0.0039	0.001
MW-12A	Trichloroethylene	96.0	5.0
	1,2-Dichloroethylene	69.0	5.0
MW-12B	Trichlorethylene	17.0	1.2
	1,2-Dichloroethylene	8.7	1.2
MW-13A	Trichlorethylene	670.0	20.0
	1,2-Dichloroethylene	ND	20.0
	Aroclor 1260	0.023	0.005
MW-13B	Trichloroethylene	11.0	0.25
	1,2-Dichloroethylene	ND	0.25
	Aroclor 1260	0.0018	0.001
MW-13B Dup	Trichlorethylene	37.0	1.2
	1,2-Dichloroethylene	24.0	1.2
	Aroclor 1260	0.0011	0.001
MW-14A	Trichloroethylene	9.6	0.25
	1,2-Dichloroethylene	ND	0.25

MDL = method detection limit; ND = not detected

Table 6-1: PCB Chemical/Physical Properties*

Property	Value
Molecular Weight	324 to 460, average = 370 g/mole
Appearance	light yellow sticky resin
Henry's Law Constant	0.0071 atm·m³/mol
Log K _{OC}	6.42
Log K _{OW}	6.91
Solubility in Water	0.080 mg/L at 24°C
Vapor Pressure	4.0 x 10 ⁻⁵ mm at 25°C

Table 6-2: TCE Chemical/Physical Properties*

Property	Value
Molecular Weight	131.4 g/mole
Henry's Law Constant	0.01 atm·m³/mol
Log K _{OC}	2.0
Log K _{OW}	2.4
Solubility in Water	1100 mg/L at 25°C
Specific Density	1.46
Vapor Pressure	74 mm at 25°C
Biodegradation Rate (1/day)	0.0014

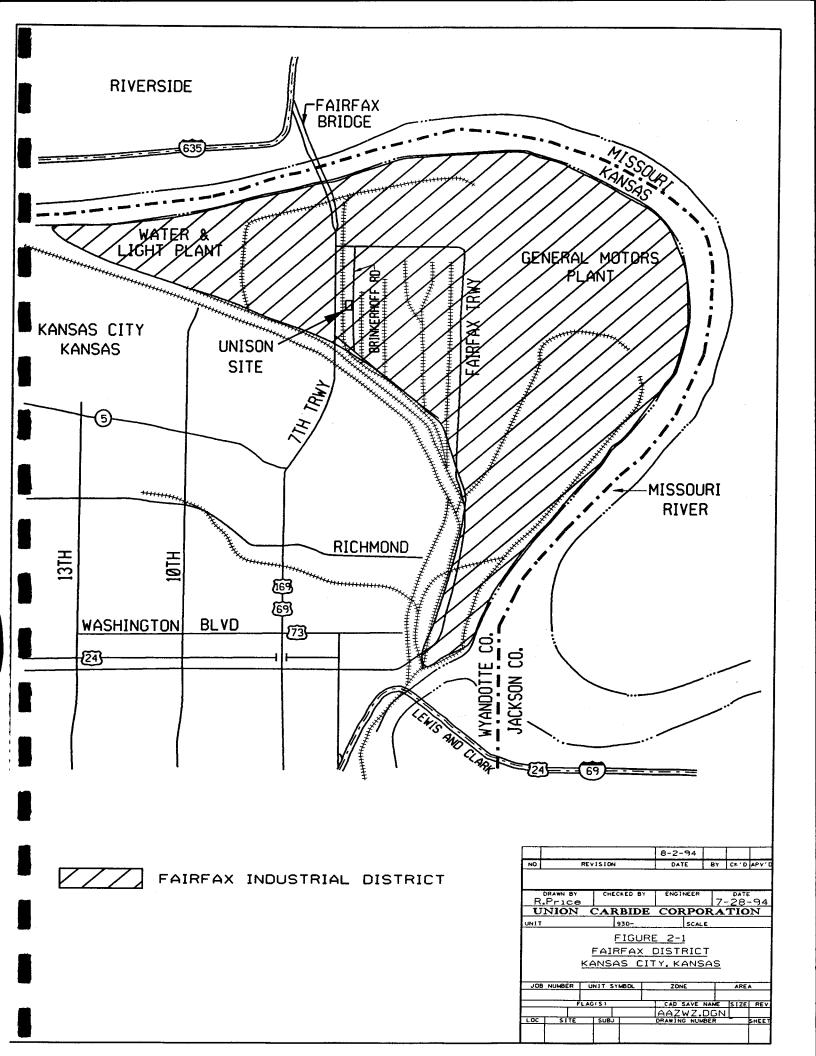
^{*} from Montgomery and Welkom (1990)

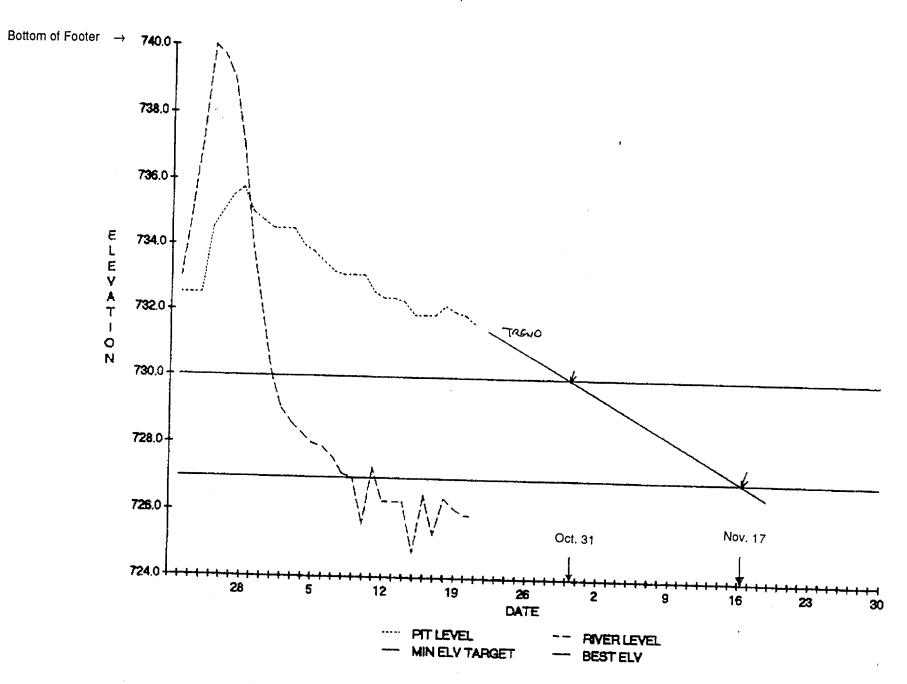
Table 7-1: RISKPRO, AT123D Model Inputs

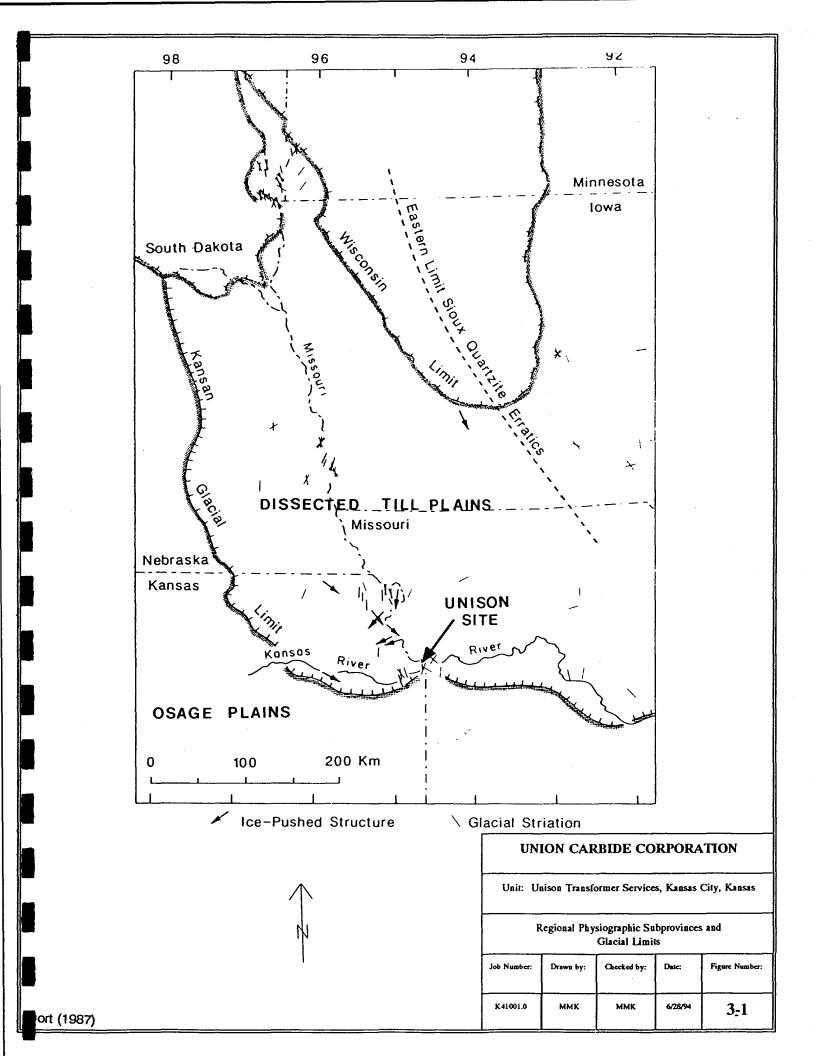
Source	Continuous
Aquifer Width	Infinite
Aquifer Depth	24.4 m
Source Width	3.05 m
Source Length	15.55 m
Hydraulic Gradient	0.000758 ft/ft
Hydraulic Conductivity	0.36 m/hr
Effective Porosity	0.28
Longitudinal Transmissivity	4
Transverse Transmissivity	4
Vertical Transmissivity	1
Soil Organic Carbon Content	0.1%
Model Run Time	75 yrs
K _D - TCE	0.0001 m ³ /kg
K _D - PCB	$0.00526 \mathrm{m}^3/\mathrm{kg}$
Molecular Diffusion (TCE and PCB)	0 m ² /hr
Decay Constant - TCE	0.0000593 /hr
Soil Bulk Density	1350 kg/m ³
Release Rate* - TCE	0.015 kg/hr
Release Rate* - PCB	0.00015 kg/hr
PCB:TCE at Source Release	1:100

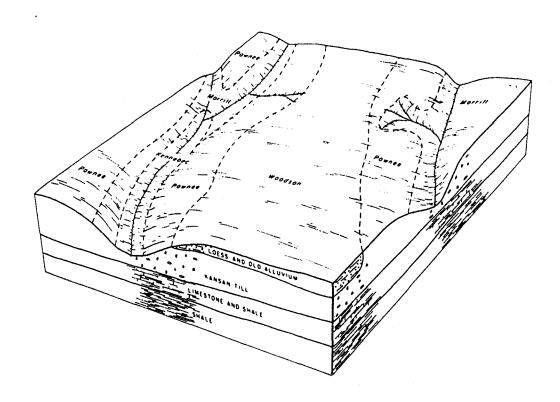
^{* 1983} to 1994

FIGURES









UNION CARBIDE CORPORATION

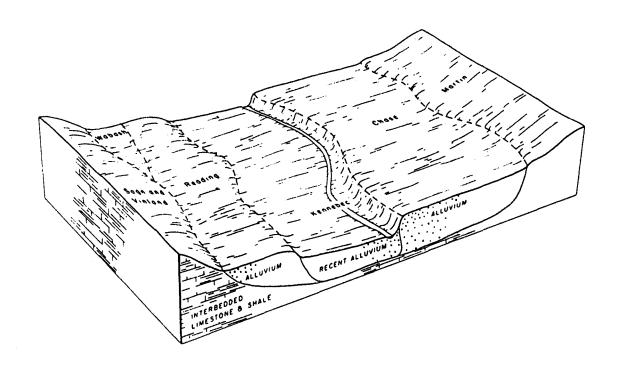
Unit: Unison Transformer Services, Kansas City, Kansas

Regional Soil Profile

 Job Number:
 Drawn by:
 Checked by:
 Date:
 Figure Number:

 K41001.0
 MMK
 MMK
 6/28/94
 3-2

Plinsky et al. (1979)

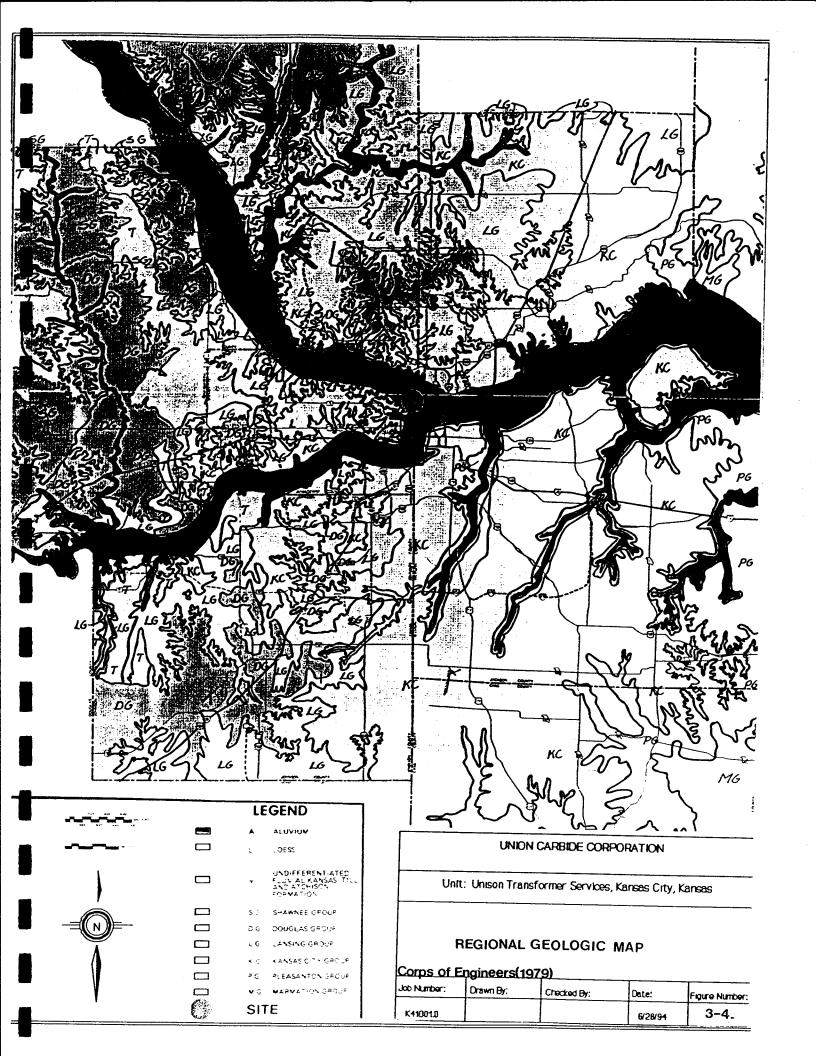


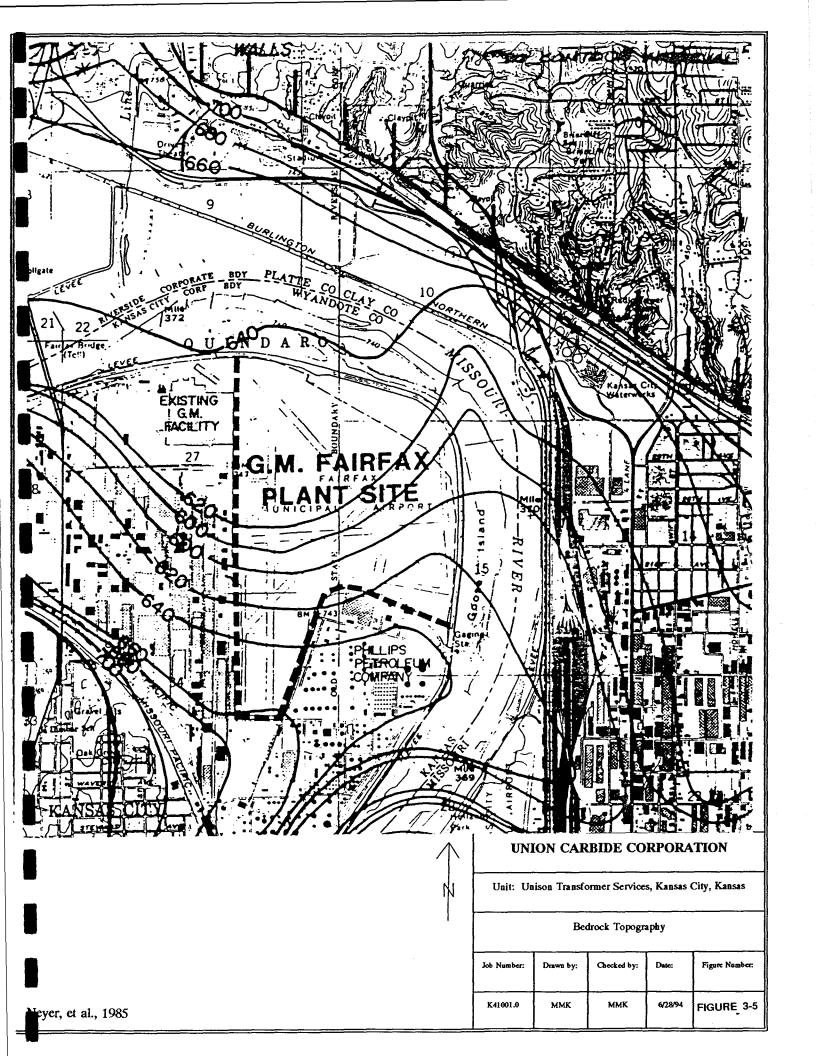
UNION CARBIDE CORPORATION

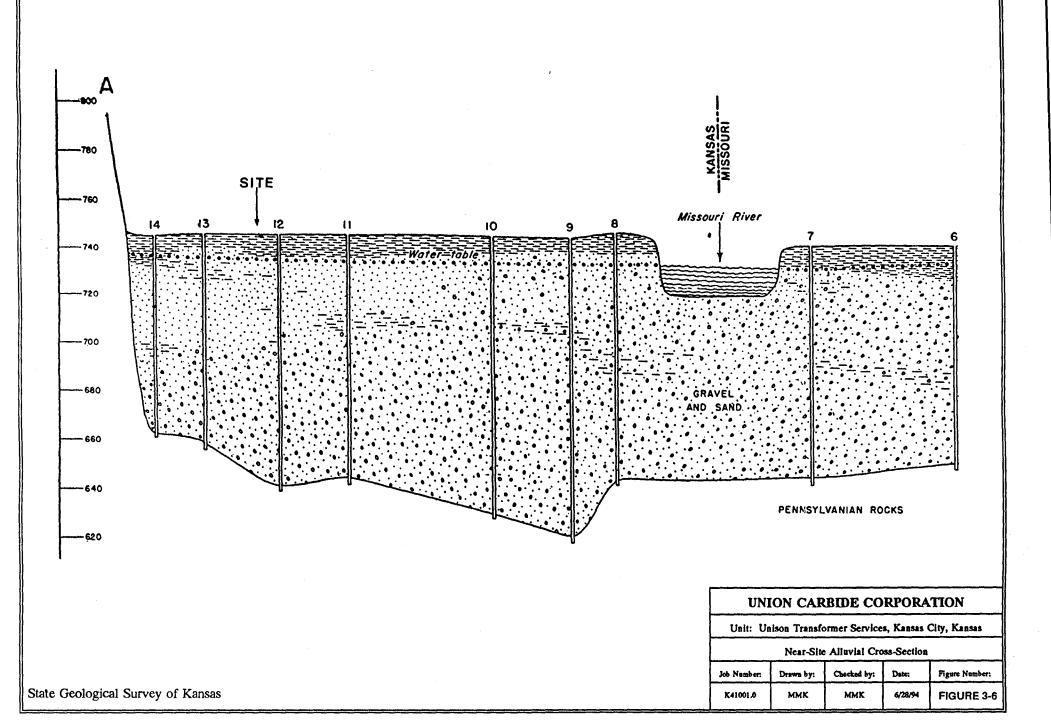
Unit: Unison Transformer Services, Kansas City, Kansas

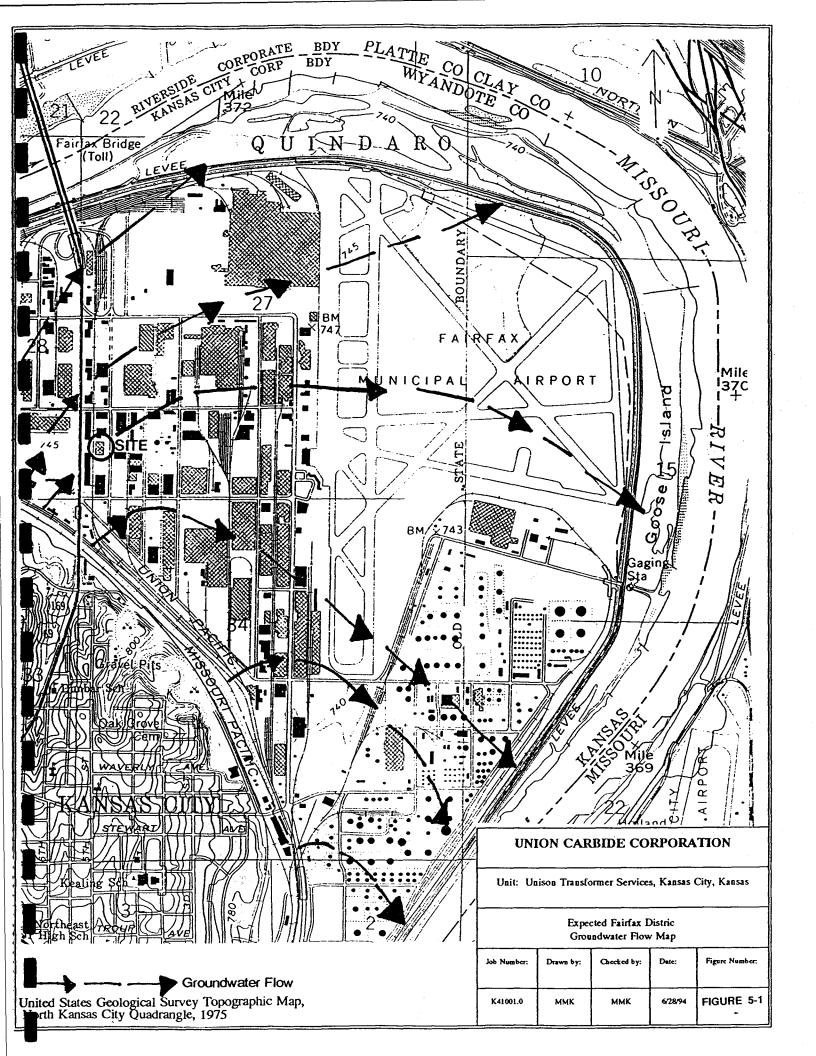
Alluvial Soil Profile

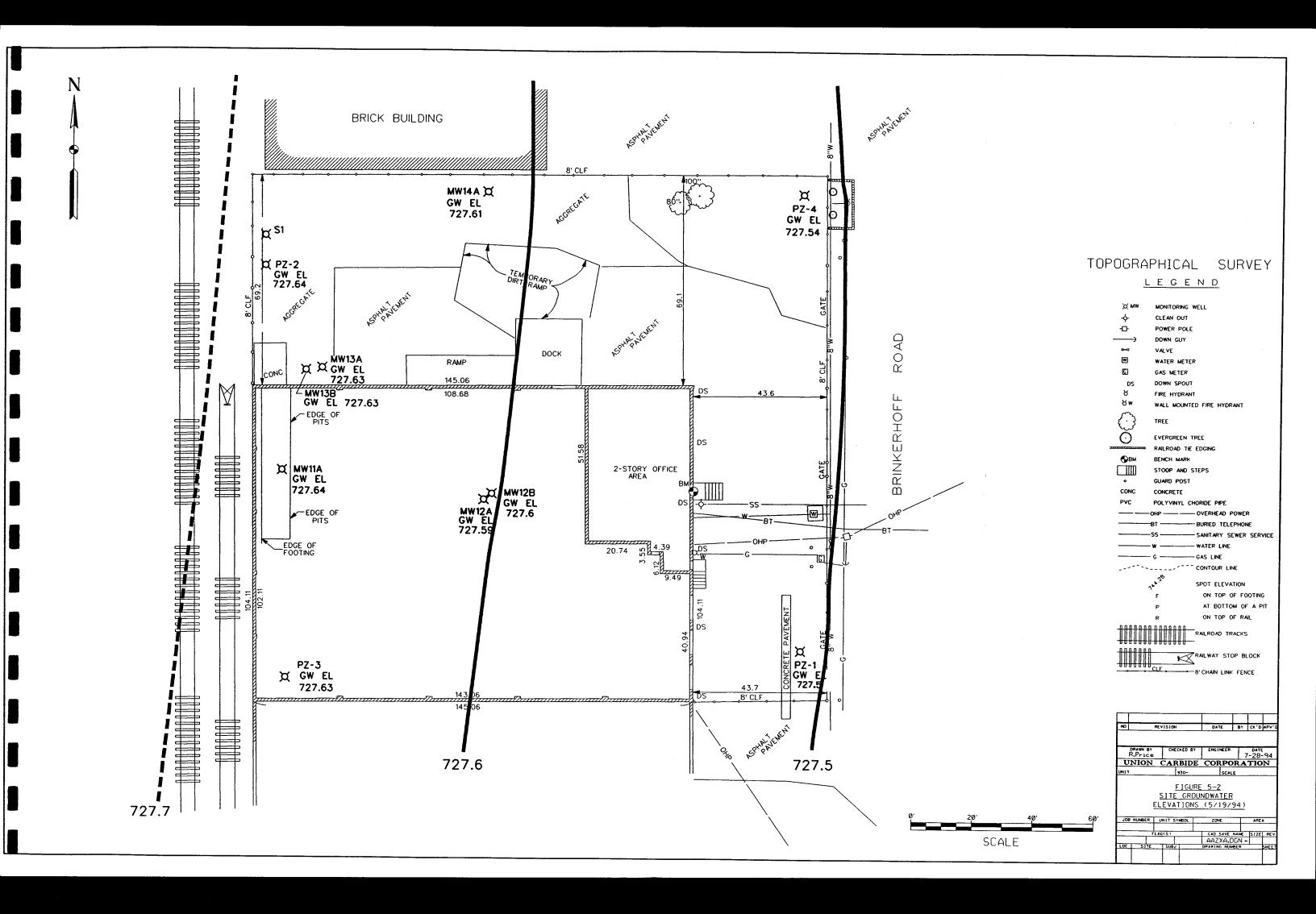
Job Number:	Drawn by:	Checked by:	Date:	Figure Number:
K41001.0	ммк	ммк	6/28/94	3-3

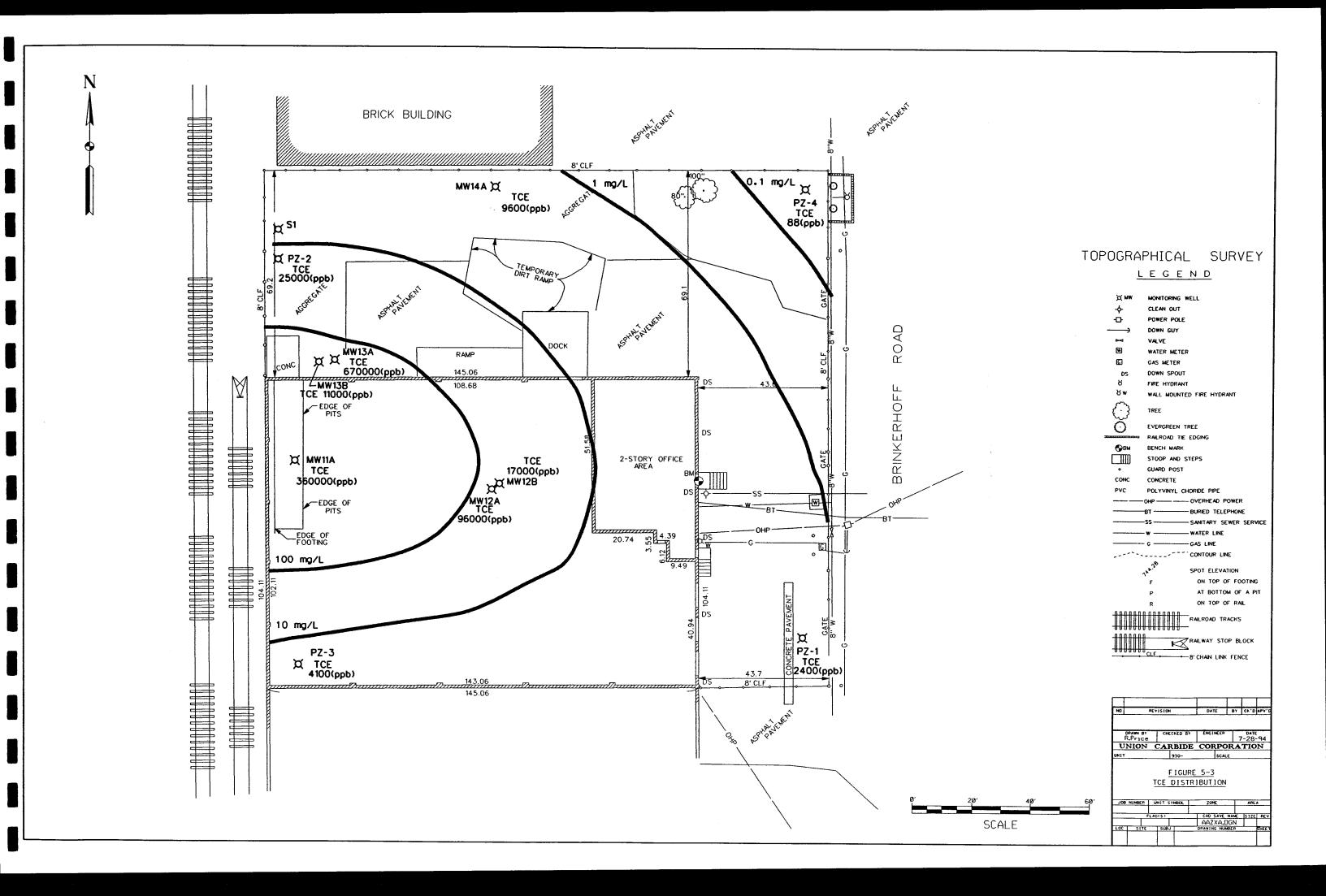


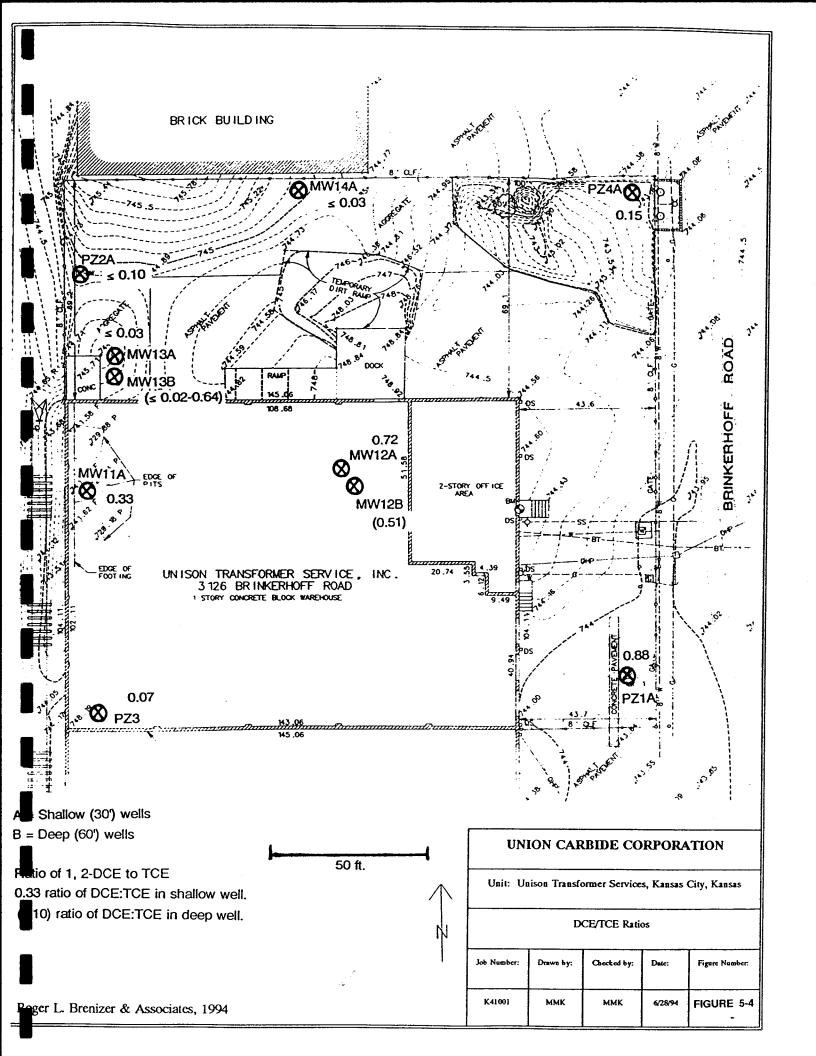




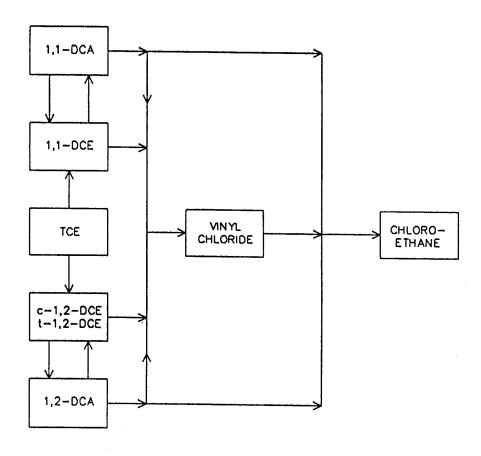




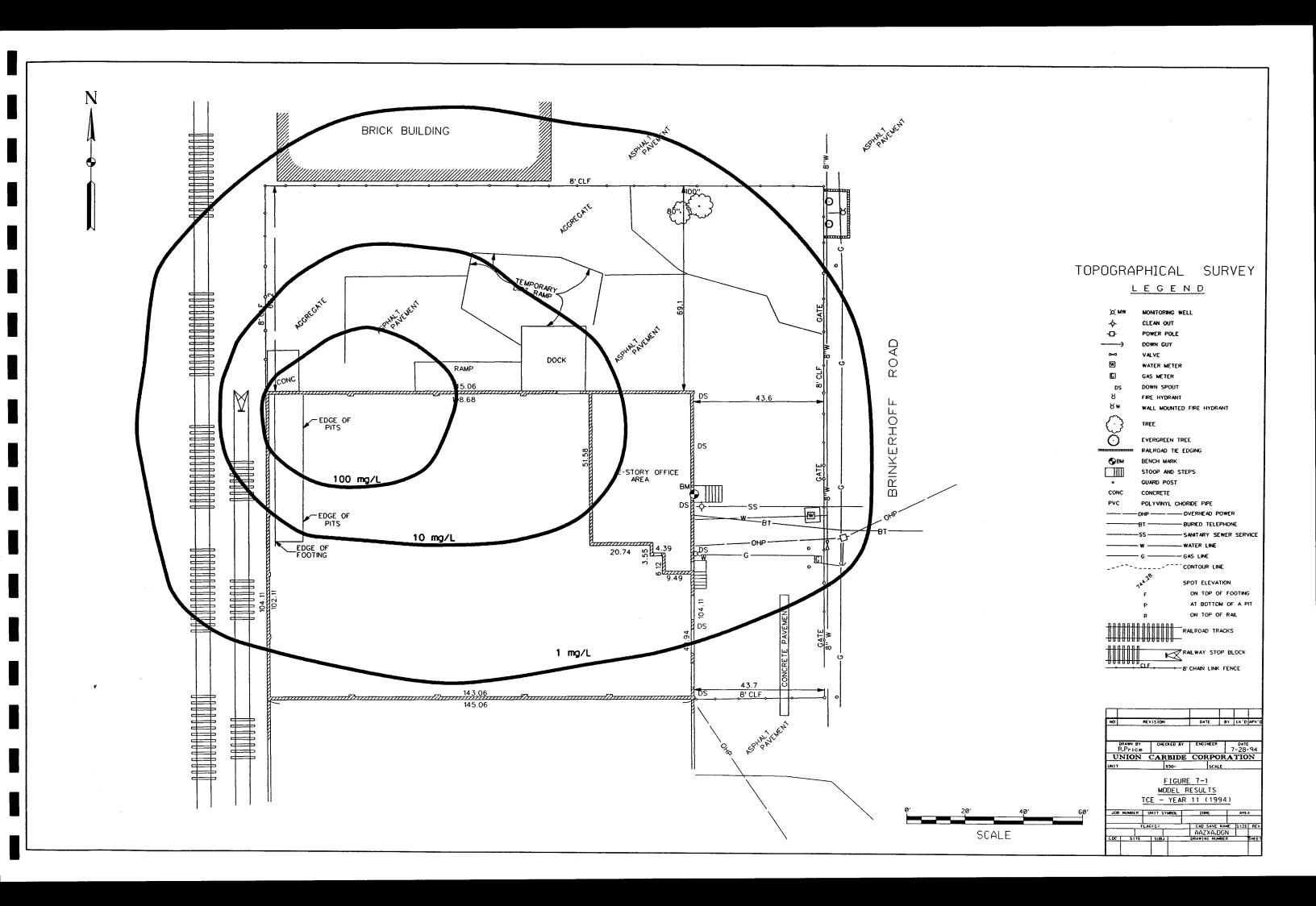


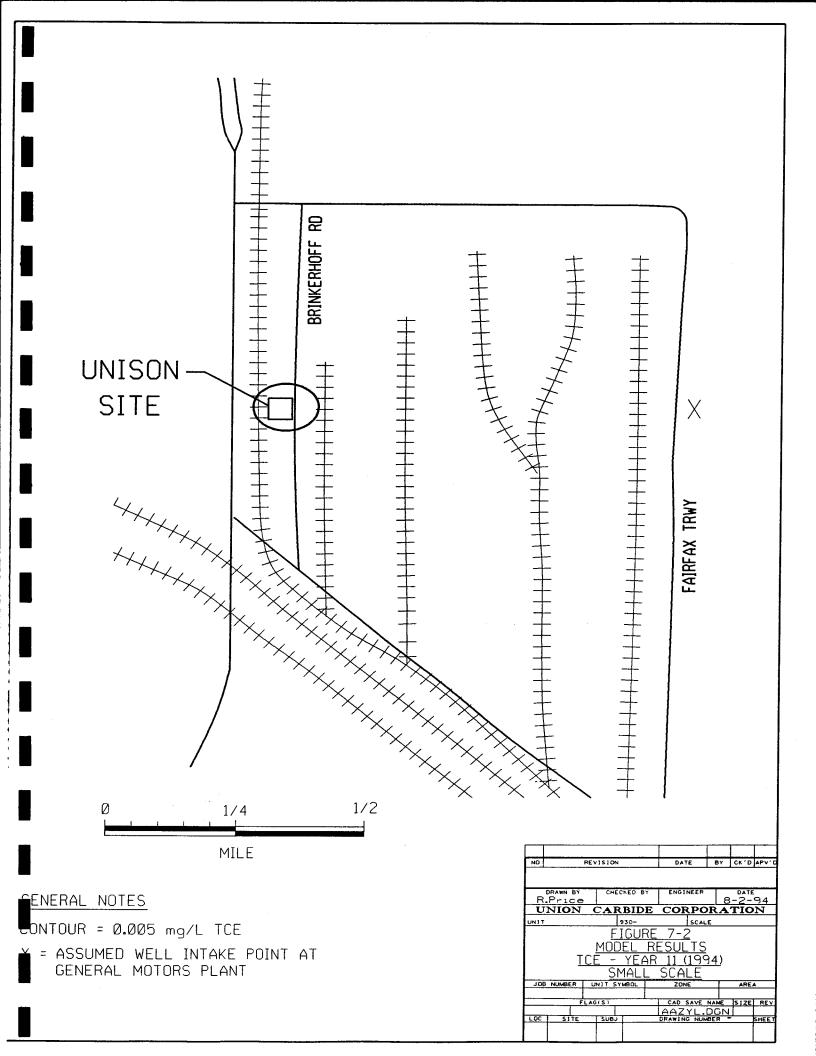


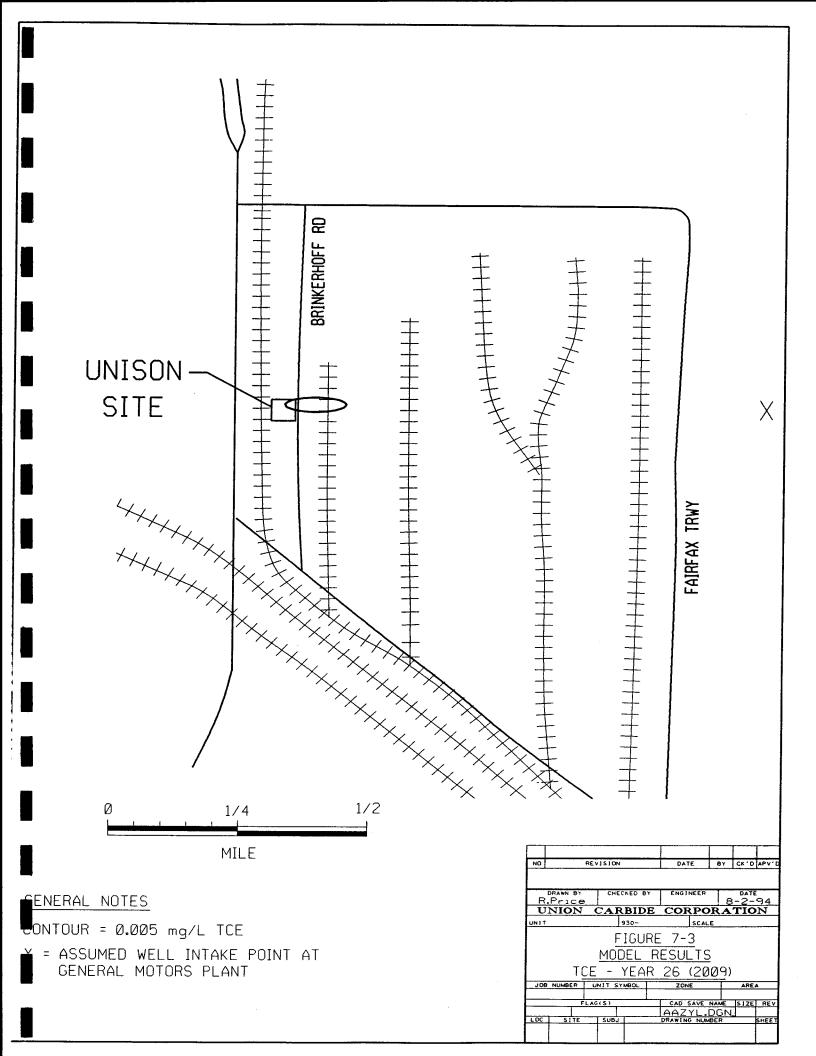
TCE DEHALOGENATION

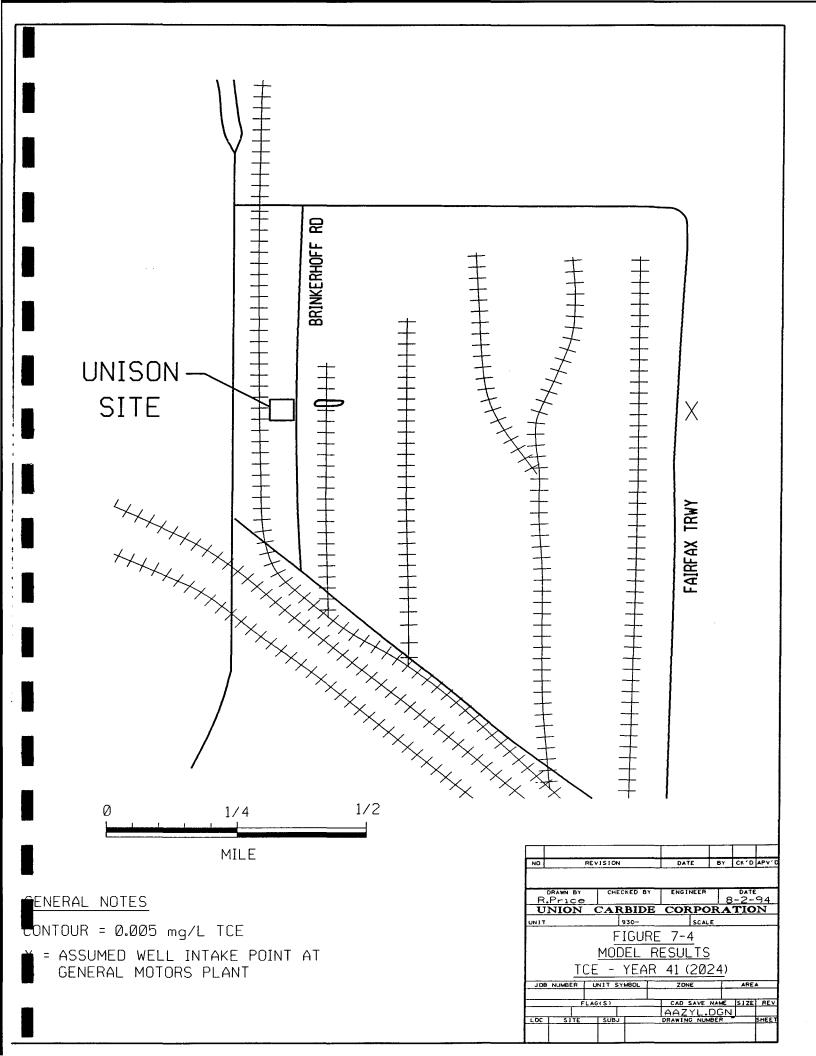


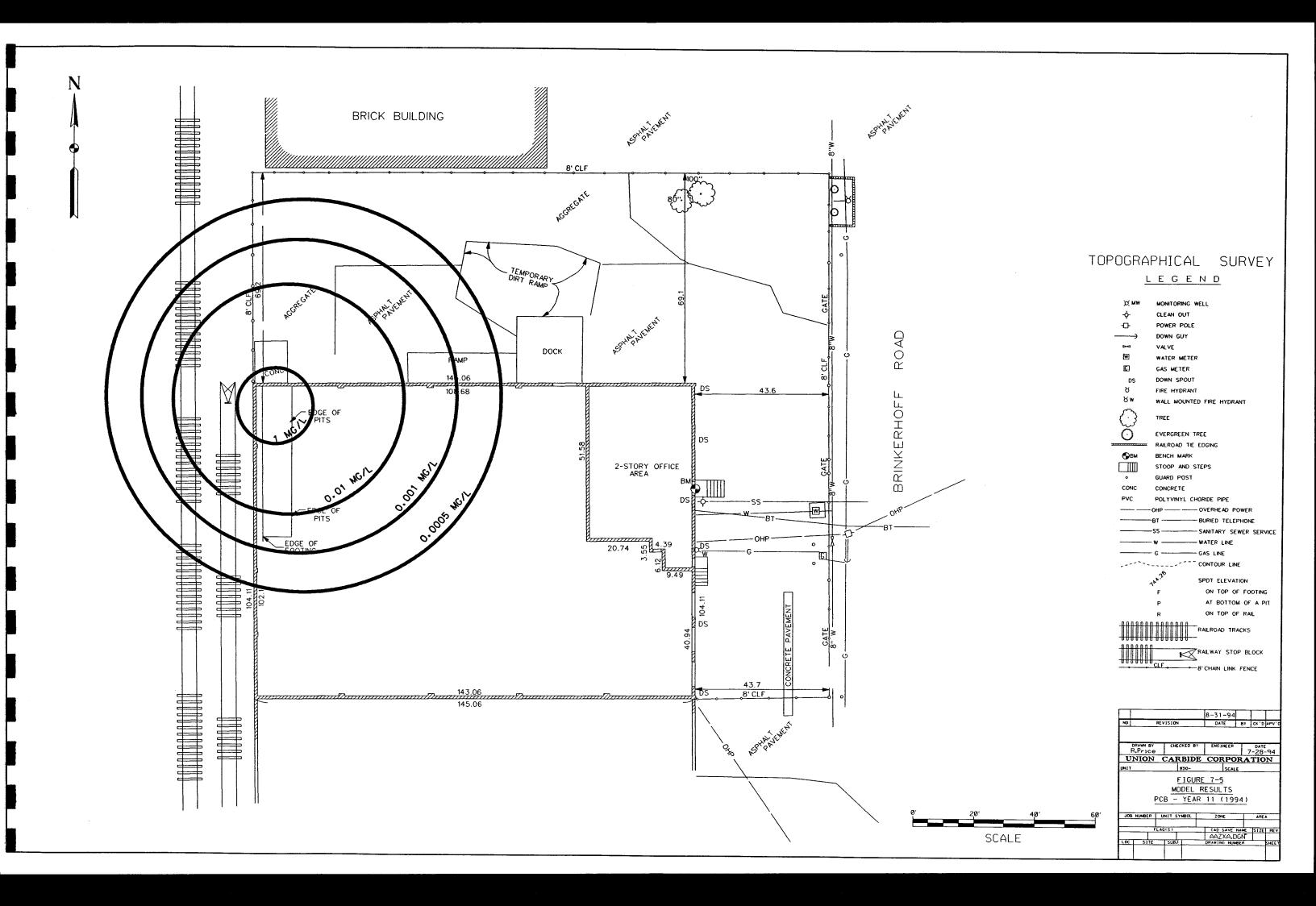
Transformation pathways for various chlorinated volatile hydrocarbons in soil systems.

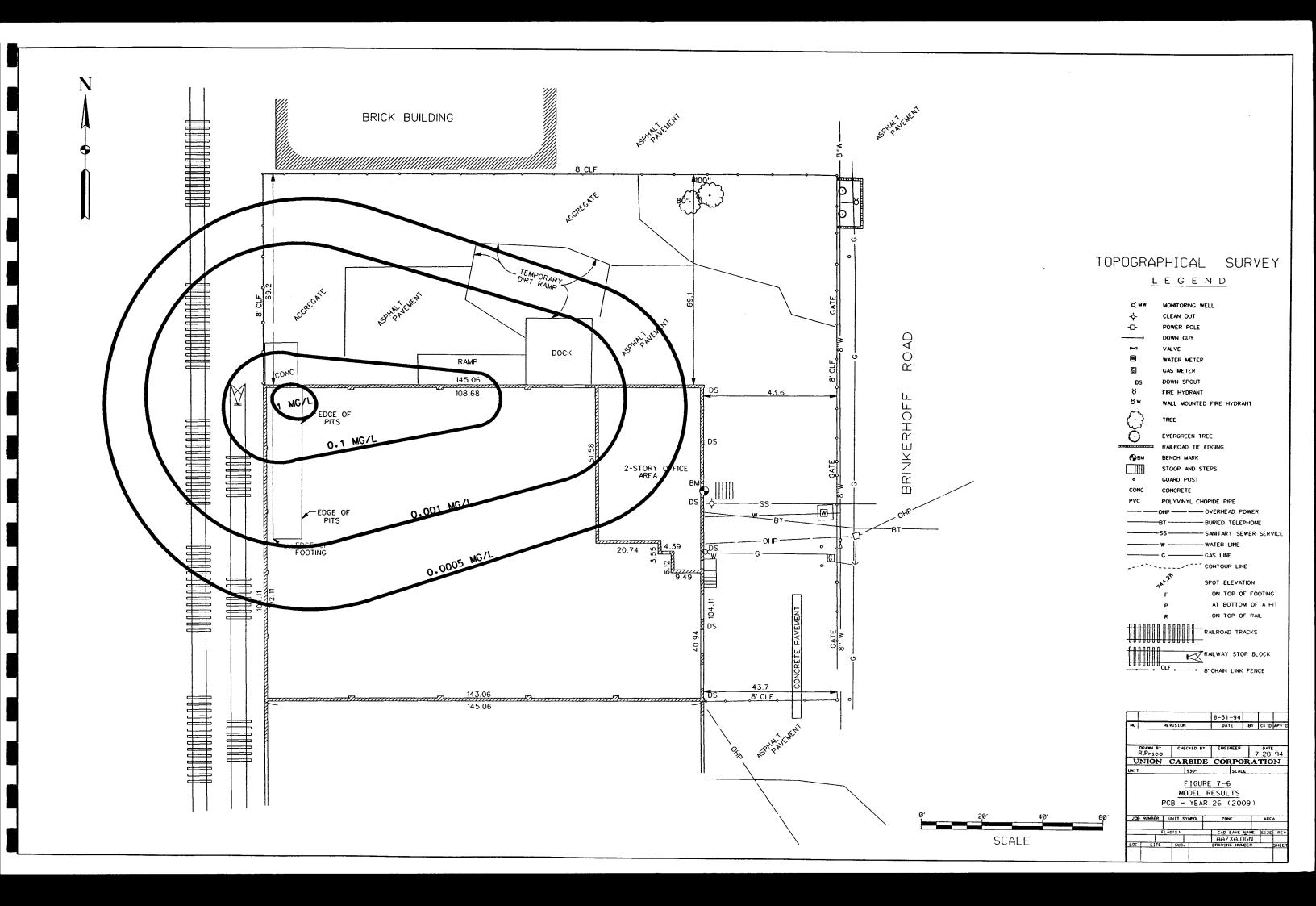


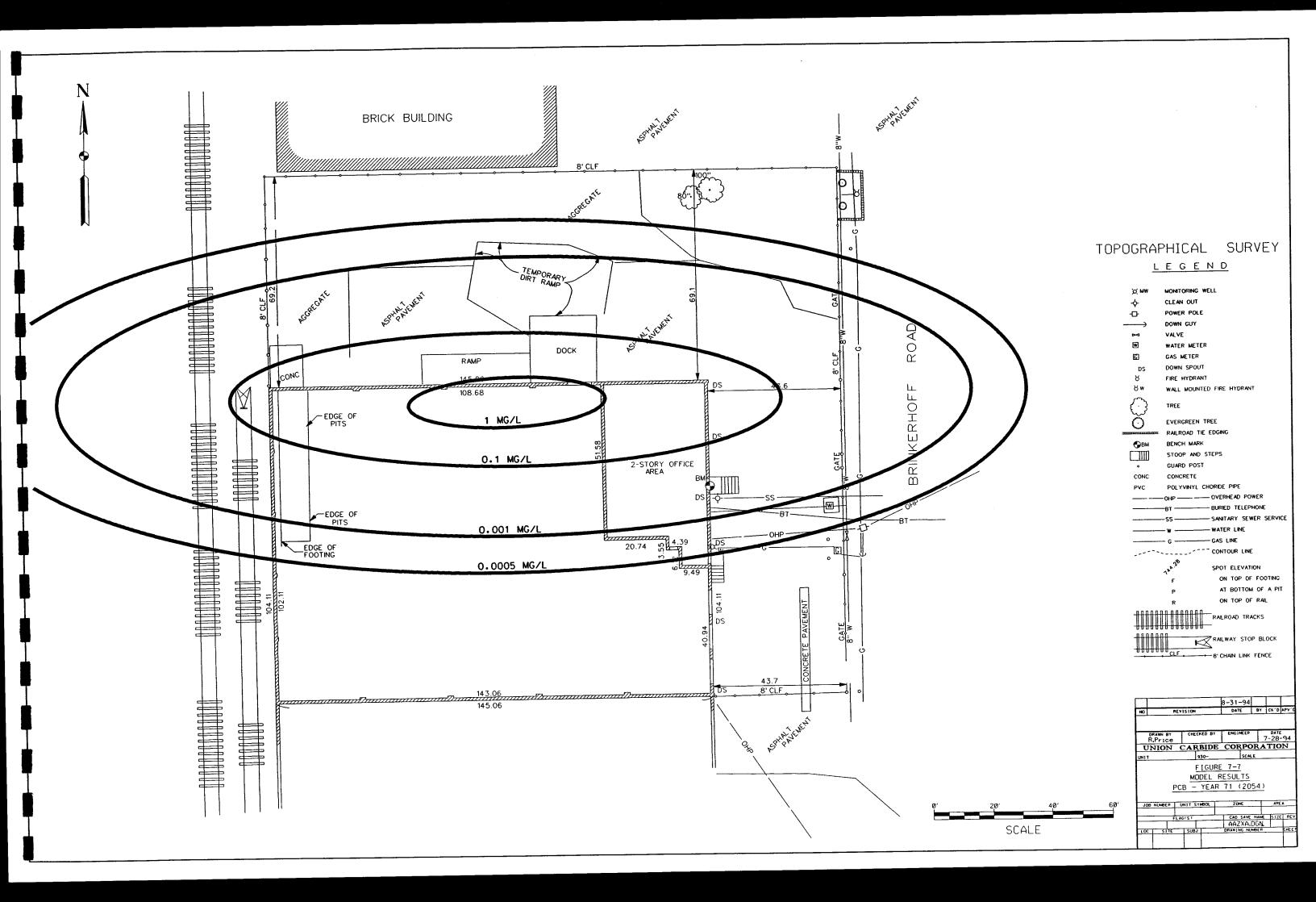




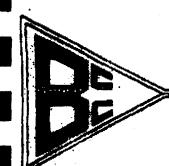








APPENDIX A



BOWDEN

RCUD 3/15/94

CONTRACTING COMPANY, INC.

1030 PAWNEE AVENUE KANSAS OITY, KANSAS 66105

March 14, 1994

913-342-5112 FAX 913-342-5145

Mr. Eric Radko Westinghouse Remediation Services, Inc. 21750 Cedar Avenue P.O. Box 550 Lakeville, MN 55044

RF: Unison Project 3126 Brinkerhoff Road Kansas City, Kansas

Dear Mr. Radke,

On Friday, March 11, 1994. Ken Blom of Kenneth M. Blom & Associates (structural engineer) and I, met with Roy Leonard of Alpha-Omega GeoTech, Inc. (soils engineer) at Roy's office to discuss the current situation at the referenced sits and the possibilities of excavating deeper as you requested.

Roy had received a packet of information of the helical underpinning on March 7, 1994 to allow him time to review the method Ken was proposing to use. When Ken and I presented to Roy our ideas of what we were proposing to do, Roy had grave concerns of the current situation. Roy had concerns about any additional excavating occurring and commented that we were "treading on thin ice as it stands right now". Roys concerns are as follows:

- 1. Bearing failure of footings occurring due to the saturated sand getting wetter due to rising water table or surface water migration. There is a high possibility in the present state, excavated 13 feet below bottom of footing that the wet sands will flow and result in voids in all directions.
- 2. Basal failure of the interior column foundations due to the saturated sands, these could be undermined easily.
- 3. This is the time of year where historically we have the greatest chance of rains. According to Mark Liggett of Unison, the water table has risen approximately eight inches in the last week.

Mr. Bric Racke March 14, 1994 Page 2

- If the three (3) cells are maintained open, immediately he would recommend a dewatering strategy.
- For us to make any recommendations due to the seriousness of the existing conditions, a couple of things need to happen.
 - A. Soils investigation to determine soils bearing and soils makeup to allow for proper structural recommendations.
 - B. Dewatering of the surrounding area to a depth that would allow for the soils to dry out and stabilize.

As it stands right now, in light of my last meeting with Ken Blom and Roy Leonard, I feel that we need to set up a meeting with you, a Unison representative, Ken and Roy and determine immediately what direction we go. Or we need to fill and compact the open cells with the clean dirt on site to head off any potential problems.

Please let me know what direction you want to take. I anxiously await your response.

Sincerely, Bowden Contracting Company, Inc.

Bowden Con

3/15/94

Par Discussion With Repared President

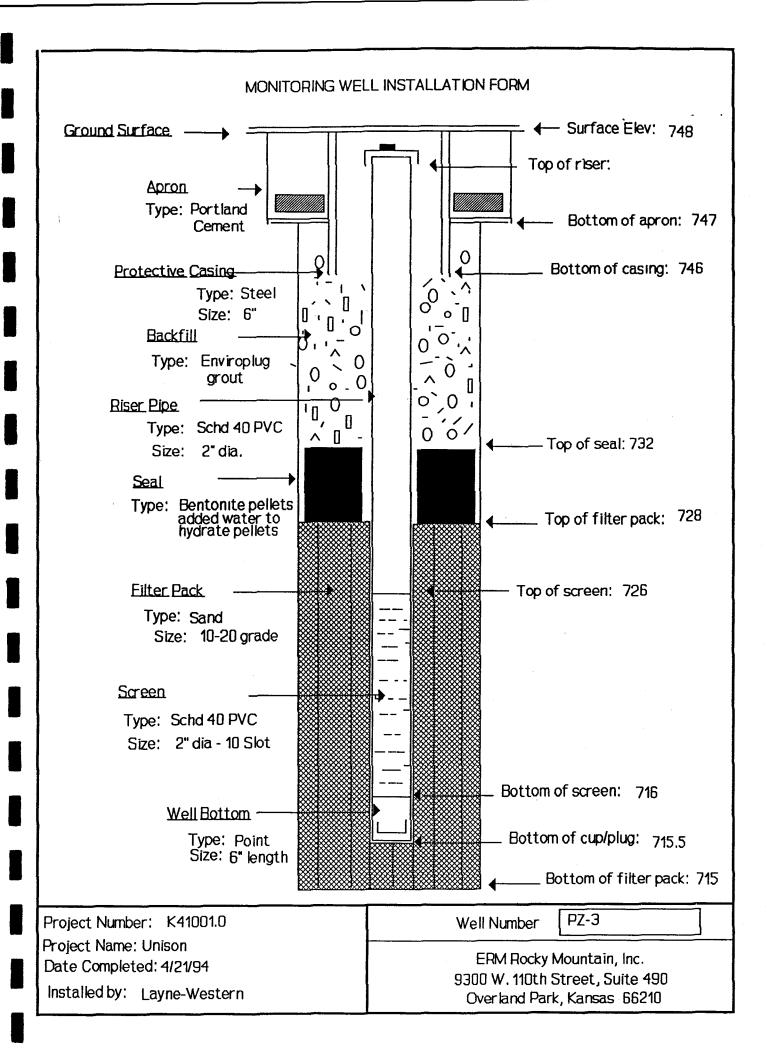
Par Rooke Assed For of these

Ren Compact to Good of these

Assed Compact to Good of the Good of t

DB: kmb 1tr/3

APPENDIX B



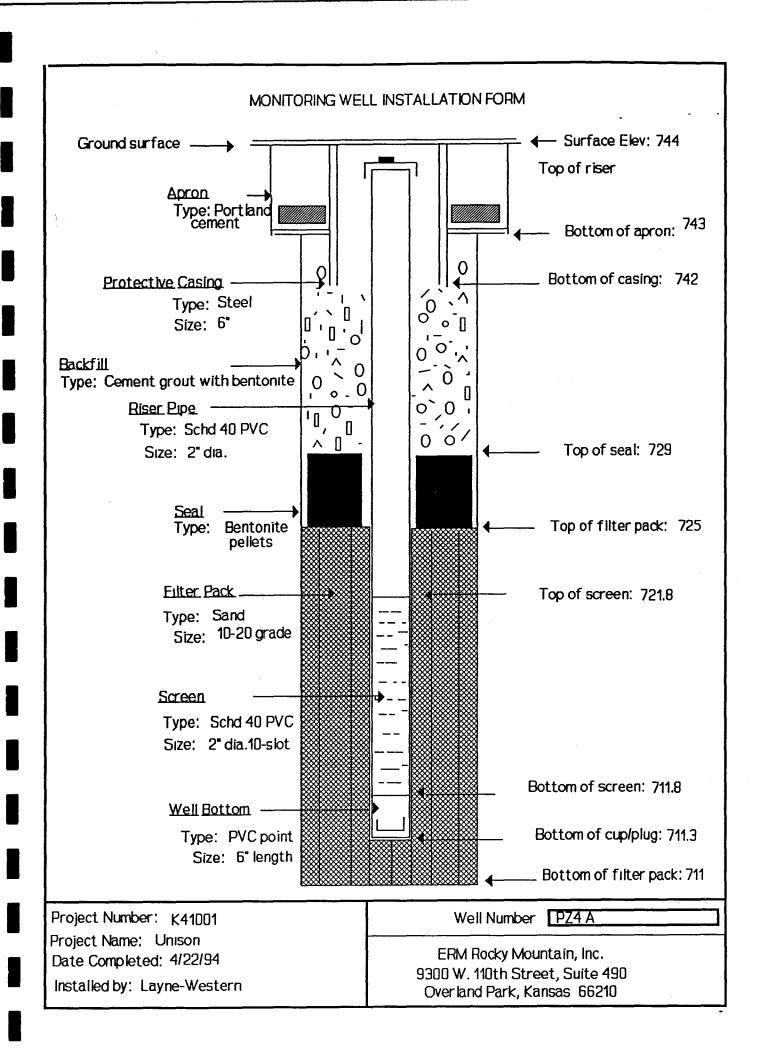
Enviror	nmen	tal Re	sources	Management	Drilling Log					
Project_	(Unison		Owner Union Carbide Corp.	Sketch Map					
Location	Kai	nsas C	ity, KS	W.O. NumberK41001.0						
Number	F	PZ-3	_Total	Depth <u>33ft,</u> Diameter <u>8" OD</u>	h					
				24hrs	Bidg.					
Screen: Dia 2" Length 10 ft. Slot Size 0.010" \boxtimes pz-3										
	Casing: Dia 2" Length 22" Type Schd 40 PVC Notes									
			ayne we	estern Drilling Method_HSA						
Driller_C				Log By MM KatzmanDate 4/21/9						
Depth (Feet)	Dings Price	Well	Sample	Description/Soil Classif (Color, Texture, Struc						
	9	্র	N 2	Concrete, gravel subgrade FILL.	Boring advanced with					
				· · · · · · · · · · · · · · · · · · ·	4 1/4" ID (8" OD) HSA. Mobile B31 mounted on					
				Very stiff, brown Silty Clay FILL with gravel. (Fil	LL) Ford farm tractor.					
- 2										
_ 3										
- 4				A feethard Clay 51th to Sulty Clay Ell 1						
- 5				Motthed Clay Silt to Silty Clay FILL light and dark brown. (F	ILL)					
- 6										
'				Predominantly Silty FILL.	(FILL)					
- 8										
9										
- 10				·						
- 11										
- 12										
L_13] L					Page <u>1</u> of <u>3</u>					

Number PZ-3

Depth (Feet)	Graphic Log	Well	Sample	Description/Soil Classification (Color, Texture, Structures)	-
13		8		SAME: Clayey Silty FILL. (FIL	L)
14				Soft Clayey SILT, wet saturated, gray-brown. Mottled light to dark.	(ML)
<u> </u>					
— 16					
<u> </u>					
- 18				Less mottling.	
19				Fine-very fine SAND, silt content decreasing.	(SM)
_ 20					
-21					
—22				Fine-medium SAND. (SP)	
23					
—24				Very damp and wet.	
<u> </u>				Brown, loose Silty SAND. (SM) Easy drilling advance.	
26					
-27					
- 28				Medium grained, poorly graded SAND with fine sand. (SP)	
— 29					,
30					Page 2 of 3

Number__PZ-3

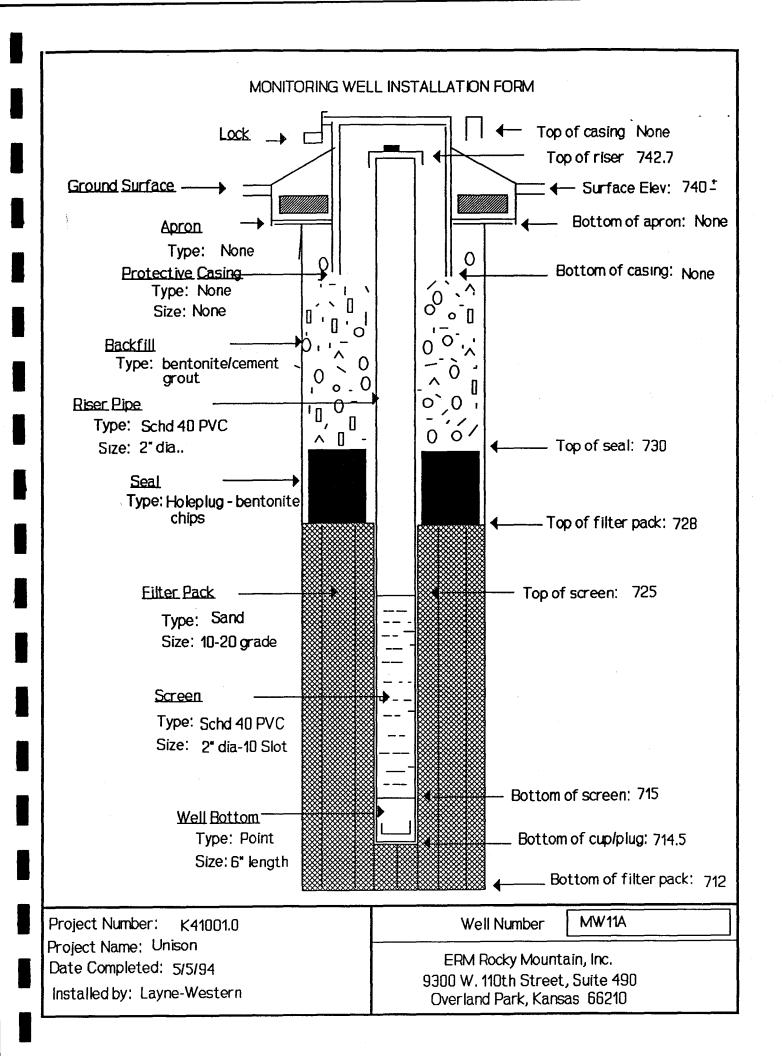
Depth (Feet)	Graphic Log	Semple Number	Description/Soil Classificati (Color, Texture, Structure	
31			SAME: Medium grained poorly graded SAND with fine SAN Harder drilling.	ID. (SP)
32			Very fine grained Silty SAND mixed with Sandy SILT.	(SM)
— 33				Bottom of Boring 33 ft.
_				
_				
-				
_				
_				



Environmental Resou	rces Management	Drilling Log
ProjectUnison	Owner Union Carbide Corp.	Sketch Map
	KSW.O. NumberK41001.0 otal Depth _32 ' Diameter8" OD	Gale Brinkerhoff
Surface Elevation 744	Water Level: Init 24 hrs	Bidg.
Screen: Dia_2"	_Length10	
	Length 20 Type Schd 40 PVC e Western Drilling Method HSA	Notes
Driller <u>O.J. Harper</u>	Log By <u>MM Katzman</u> Date <u>4/21/9</u>	94
Depth (Feet) (Feet) Log well construction	Description/Soil Classif (Color, Texture, Struc	
_ 1 _ 2	Gravel Sand Fill. Very fine grained Silty SAND, brown to Sandy Silt low plastic saturated.	Boring advanced w/ 4-1/4" ID HSA (B" OD), w/ Mobile B 31 on Ford farm tractor, used teflon knock-out plug. (SM-ML)
_ 3	Becoming very fine Silty SAND brown, moist.	(SM)
- 5 - 6		
- 7		
- 8	Becoming damp to wet with some free water.	(SM)
- g l		
- 10		
- 11		
- 12		
		Page <u>1</u> of <u>3</u>

			Number: <u>pz4</u>	
Depth (Feet)	Graphic Log	Semple	Description/Soil Classification (Color, Texture, Structures)	
13	g - *	8 2		
			SAME: Brown, very fine grained Silty SAND, damp to wet.	(SM)
14				
— 15				
— 16				
-17			Becoming mixed very fine Silty SAND and fine SAND w/ same Sandy Zones.	(SM w/ SP)
18				
19				
20			Brown, fine grained , poorly graded SAND w/ same medium sand	(SP)
—21				
—22				
23				
-24			w/ much medium sand.	
— 25			Medium grained, poorly graded sand.	(SP)
— 26				
—27				
— 28				
—29				
	1 11	11 11		

				Numi	ber_PZ4		
Depth (Feet)	hic -	Description/Soil Classification (Color, Texture, Structures)					
A (F)	Graphic Log	Well	8 2		(Color, Texture, Structures)	-	
-30			-	SAME: Very Silty SAND	(SM)		
31							
— 32							
					Bottom of Boring 32 ft.		
— 33							
						ĺ	
						Ī	
. []							

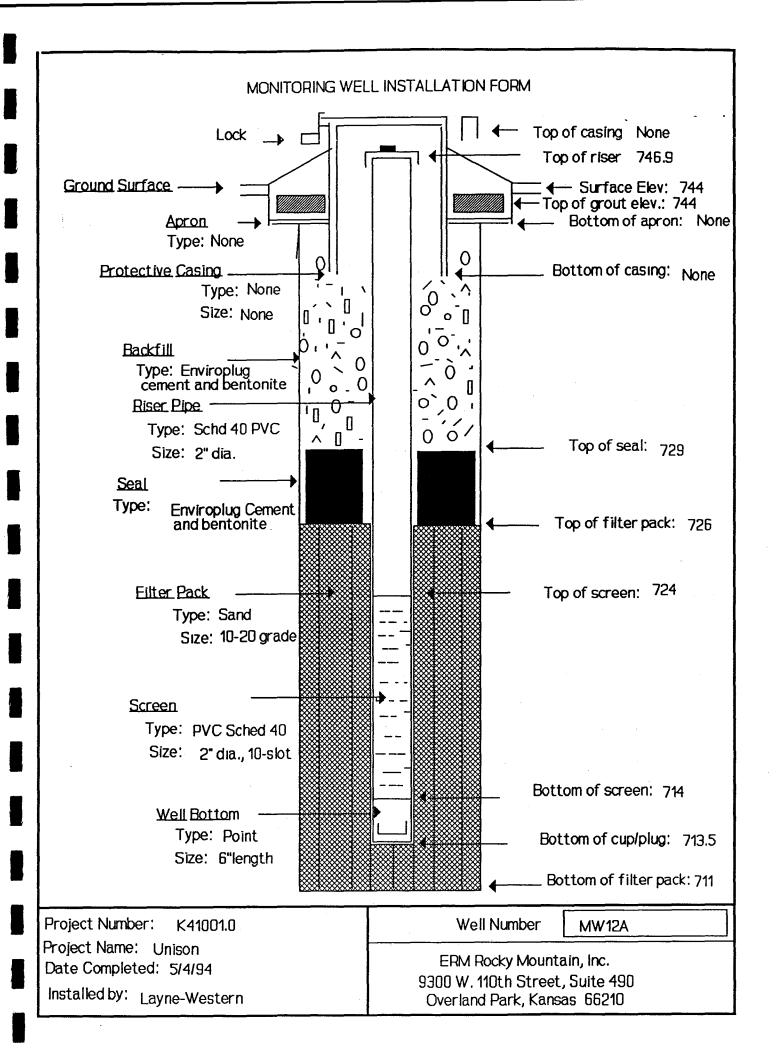


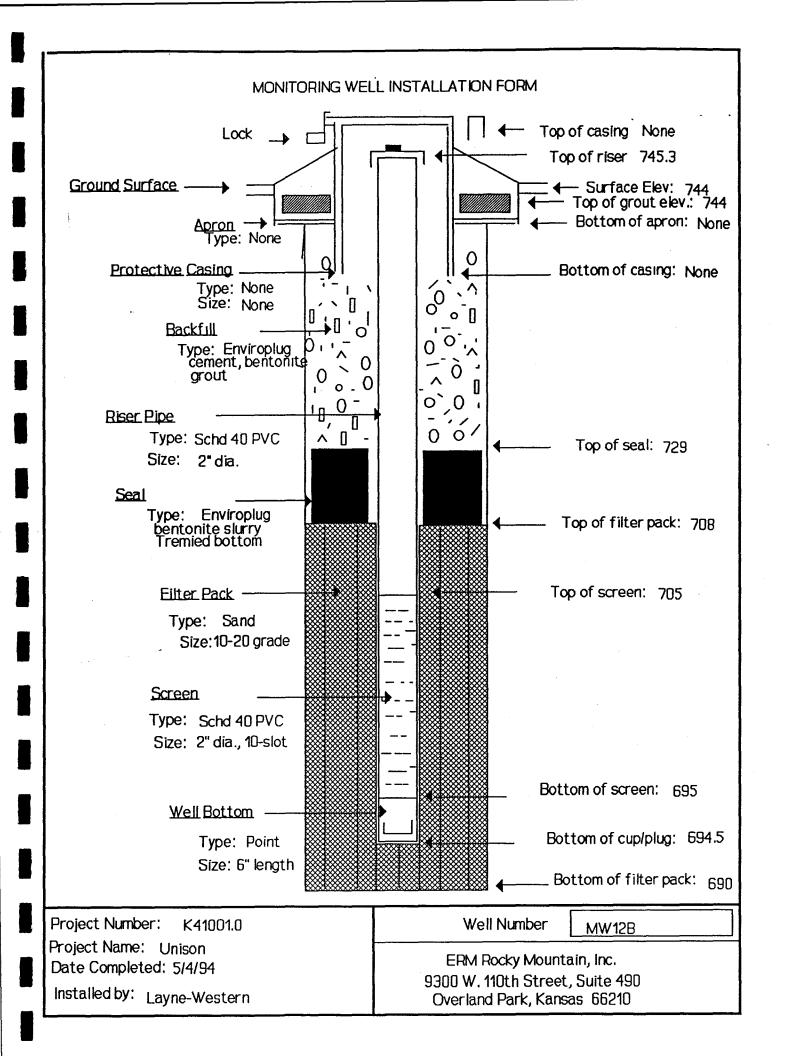
Envir	ronmen	ıtal Re	Drilling Log								
Proje	ct	Unison	Sketch Map								
L ocat	ion <u>Ka</u> Number		ity, KS /11A	W.O. Number_ <u>K41001.0</u> Total Depth <u>28</u> Diameter <u>8" OD</u>	. 1						
Surfa	Surface Elevation 741 msi Water Level: Init 24 hrs										
Scree	Screen: Dia 2" Length 10 ft. Slot Size 10 MW11A										
Casır	Casing: Dia 2" Length 18 ft. Type Schd 40 PVC Notes										
	g Comp	uiy	ayne W								
Drille	r OJ. H			Log By_ <u>MM Katzman</u> Date_5/5/94							
Depth (Feet)	Graphic Leg	Well Construction	Sample	Description/Soil Classif (Color, Texture, Struc							
_1				FILL Dense, low plastic Silty clay to Clayey Silt FILL. (File	Boring advanced with 8" OD-4 1/4" ID HSA with truck mounted Acker 82						
- 2					•						
_ 3											
- 4											
- 5											
- 6											
7											
- B											
- g											
- 10				With OCC gravel sized rock							
- 11				fragments.	(FILL)						
- 12											

Page <u>1</u> of <u>2</u>

Number MW11A

Log Well	Sample	Description/Soil Classif (Color, Texture, Struc	
- ⁸		SAME: FILL.	(FILL)
		Very fine, low damp-moist, plastic Clayey-Sandy SILT	. (ML)
		Very fine grained Silty SAND saturated.	(SM)
		•	
		Becoming fine with some SILT.	(SM-SP)
		Dark brown, medium, poorly graded SAND.	(SP)
		Fine-medium, poorly graded SAND	(SP)
		Light brown, very fine Silty SAND.	(SM)
			Bottom of Boring 28ft.
			Detail of Doring Lore
		Log Log Construction Sample Number	SAME: FILL. Very fine, low damp-moist, plastic Clayey-Sandy SILT Very fine grained Silty SAND saturated. Becoming fine with some SILT. Dark brown, medium, poorly graded SAND. Fine-medium, poorly graded SAND



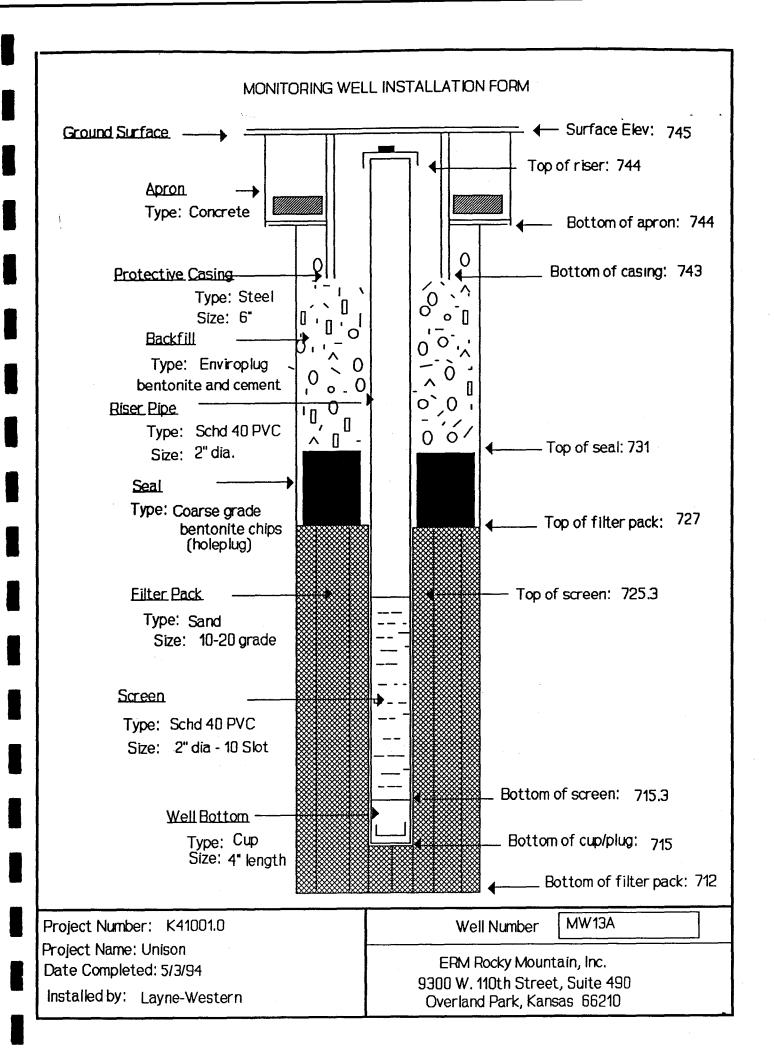


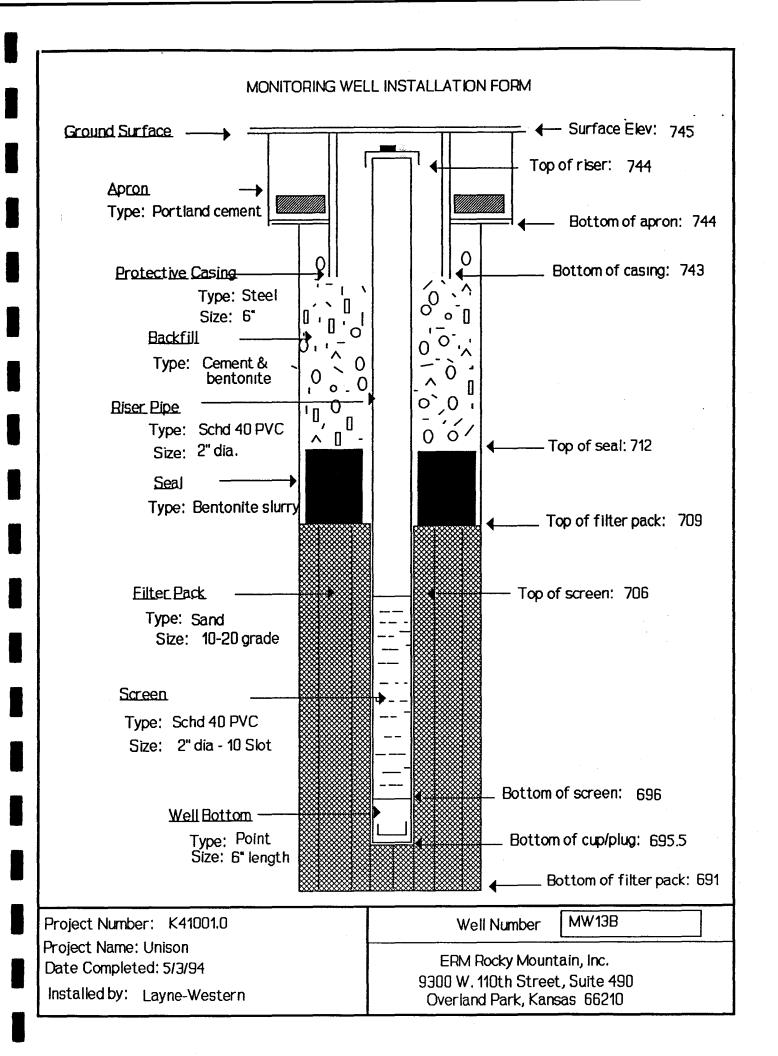
Enviro	o n men	tal Res	sources	Management	Drilling Log						
Projec	:t	Unison	·	Owner Union Carbide Corp. Sketc	h Map						
				W.O. Number K41001.0	. 1						
	Number MW12B Total Depth 54 Diameter 8 OD Surface Elevation 744.5 msl Water Level: Init 24 hrs On MW12B										
Screen: Dia 2" Length 10' Slot Size 10											
Casing	Casing: Dia 2" Length 40' Type Schd 40 PV Drilling Company Layne Western Drilling Method HSA										
	-			Log By MM Katzman Date 5/4/94							
Depth (Feet)	Graphic Log	Well Construction	Sample	Description/Soil Classification (Color, Texture, Structures)							
_ 1				FILL.	Boring advanced by 8" OD HSA w/ truck mounted Acker 82.						
- 2				Light brown-tan, dry, very fine Clayey SILT. (ML)							
_ 3											
- 4				Becoming damp.							
- 5				Mixed low plastic, Silty CLAY and slightly sandy, Clayey SILT	. (CC & ML)						
- 6											
- 7				Low plastic, Silty CLAY, damp dark brown.	(CL)						
- 8				Low plastic, brown Clayey SILT.	(ML)						
- g											
10											
- 11											
_ 12											
_13					Page <u>1</u> of <u>4</u>						

(Feet)	Graphic Log	Well Construction	Sample	Description/Soil Classification (Color, Texture, Structures)	
- 13 - 14		8		SAME: Light brown, low plastic, Clayey SILT.	(ML)
– 15					
— 16				Light brown, very fine Silty SAND to Sandy SILT ((SM)
- 17				with dicrete zones/strata of low plastic Clayey SILT	(ML)
- 18				moist to damp.	
- 19					
- 20 -21				Becoming Sandy SILT to Silty SAND.	(SM)
-22					
_23					
-24					
- 25				Very fine grained Silty SAND with tr. medium sand.	(SM)
_ 26				SAME: very fine grained Silty SAND with tr. medium sand.	(SM)
- 27					
- 28					
-29					

Envir	onmen	itai kes	ources	Management	Drilling Log
[]	U	E		Number MW12B	
Depth (Feet)	Graphic Log	Well	Semple	Description/Soil Classification (Color, Texture, Structures)	
1 1	ا حق	Sorist 💉	"	(COO), Texture, or detailed	•
30					
31				Becoming fine SAND with some SILT.	(SP w/SM)
- 32					
— 33					
— 34					
- 35					
				Fine SAND with tr. of medium SAND with some very fine Silty	SAND. (SP w/SM)
- 36					
— 37					
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \					
—38					
-39				SAME: Fine SAND with tr. of medium SAND with some very fi	ine Silty SAND. (SP w/ SM)
40					
40					
<u> </u> 41					
42				Fine to medium SAND with tr. coarse SAND.	(SP)
'				Fine to medium Sand with the coarse Sand.	(or)
<u> </u>					
— 44					
— 45					
—46					

nviro	nmen	tal Res	ources	Management Number: MW12B	Drilling Log
ا اج	ဋ	g	₽ №	Number:MW 12B Description/Soil Classification	l
(Feet)	Log	Well	Semple	(Color, Texture, Structures)	
47 -		0		Same: Fine to medium SAND with tr. of coarse SAND.	(SP)
48				Medium poorly graded SAND w/ some fine and tr. of coarse sa	nd
49				with some lignite.	(SP)
50				Becoming coarse with some medium SAND with tr. of fine to medium gravel.	(SP)
51					
52					
53				Becoming fine to medium with some Silty SAND and CLAY G	JLLS. (SP w/ SM)
54					Bottom of Boring 54f
	. 1	-			
				·	





Envir	Environmental Resources Management Drillin									
Proje	ct	Unison		Owner Union Carbide Corp.	Sketch Map					
Locat	Location_Kansas City, KSW.O. Number_K41001.0									
Numi	Number MW13B Total Depth 54 ft. Diameter 8" OD									
	face Elevation 745 msl Water Level: Init - 24 hrs - WM138									
	Screen: Dia_2 inches_Length_101t,Slot Size_10									
	_			ength 40 ft. Type Schd 40 PVC	Notes					
				estern Drilling Method HSA						
Drilk				Log By Mike Katzman Date <u>5/3</u>	<u>194</u>					
Depth (Feet)	Graphic Log	Well Construction	Sample	Description/Soil Classi						
	6 3	> 15 0	Sample	(Color, Texture, Stru						
				FILL	Boring advanced with 4 1/4" ID HSA and Acker 82 truck mounted rig					
				Dark brown, Clayey SILT - damp to wet.	(ML)					
- 2										
_ 3										
- 4				Mixed low plastic Silty CLAY and Clayey SILT.	(ML & CL)					
- 5										
6										
7										
- в										
9										
- 10				Light brown very fine Silty SAND.	(SM)					
11										
-12				Mixed with layers of Sandy SILT.	(SM w/ ML)					
42										

Page <u>1</u> of <u>4</u>

Number <u>MW13B</u>

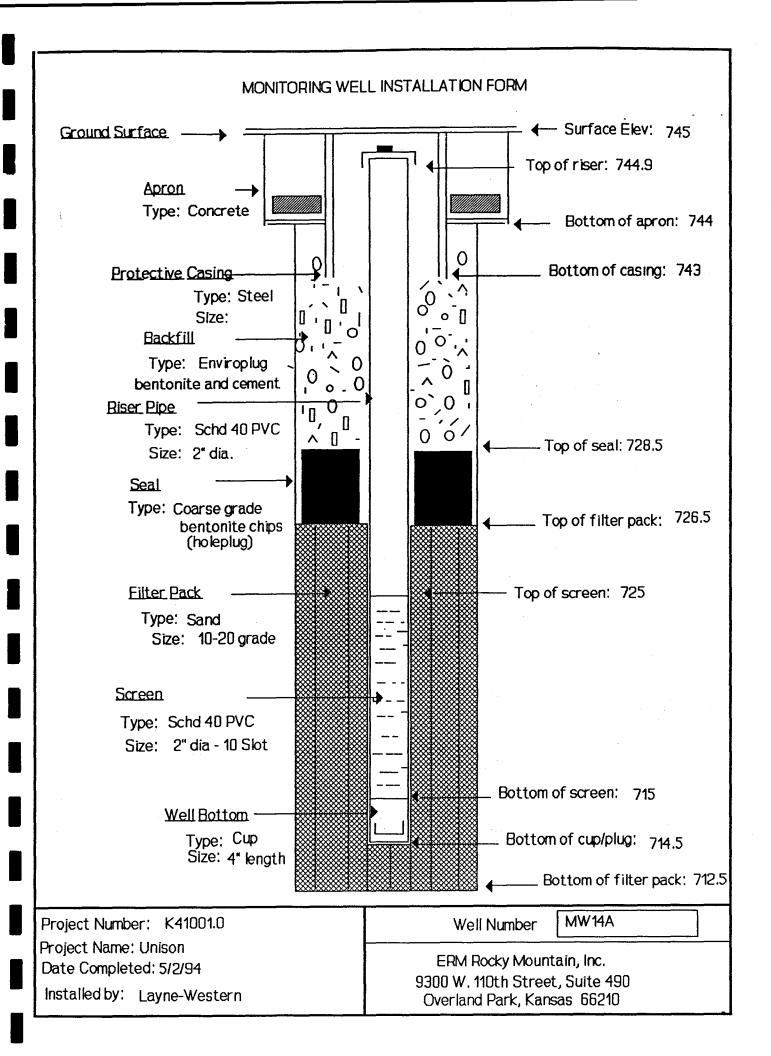
(Feet)	Graphic Log	Well	Semple	Description/Soil Classificat (Color, Texture, Structur	
- 13		8		Same: Light brown, very fine Silty SAND with Sandy Sil.	T. (SM w/ ML)
- 14 15				Light brown, very fine grained Silty SAND; moist.	(SM)
- 16					
- 17					
18				Mixed very fine Silty SAND with layers of Sandy SiLT.	(SM w/ ML)
19				Fine SAND; wet.	(SP)
- 20					
21				Fine, medium SAND with tr. of Silty SAND.	(SP w/ SM)
22					
23					
-24					
25					
26					
27					
28					·
-29				Medium grained with some fine SAND with tr. of lignite.	(SP)
, l					

Number<u>MW13B</u>

Depth (Feet)	phic c	L to	Sample	<u> </u>	Description/Soil Classification	
A 37	Graphic Log	Well	8	2	(Color, Texture, Structures)	•
- 30				-	Same: Medium grained with some fine SAND with tr. of lignite.	. (SP)
31						
— 32 —						
— 33						Elevated PID readings: Suited up to Level C.
- 34						
- 35						
— 3 6					Medium to coarse SAND with lignite; poorly graded.	
- 37						
—38						
39						
40						
—41						
— 42					Becoming medium, fine grained.	
— 43					Silty SAND. (SM)	-
—44						
— 45					No recovery.	
—46						
لحما						

Number MW13B

				Number <u>MW13B</u>	
Depth (Feet)	Graphic Log	Well Construction	Sample	Description/Soil Classif (Color, Texture, Strue	
47		8		Same: No recovery.	
- 48					
— 49				Medium, coarse grained SAND.	(SP)
— 50				Becoming medium grained SAND.	(GI)
30				becoming medical grained dans.	_
— 51					Running sands.
— 52				Becoming coarse grained SAND.	 (SP-SM)
— 53				Medium to very fine Silty SAND; poorly graded.	(5. 514)
				Medium to very fine SAND.	 (SM)
54					
_55					Bottom of Boring 54'.
				- !	



Enviro	nment	tal Res	ources	Management	Drilling Log						
Project		Unison		Owner Union Carbide Corp.	Sketch Map						
Locatio											
	Number										
Surface	Surface Elevation 745msl Water Level: Init 24 hrs Bldg.										
Screen	Screen: Dia 2" Length 10 ft. Slot Size 10 Notes										
Casing	: Dia_	2"	Len	gth20 ftType_Schd 40 PVC	Notes						
_		•		estern Drilling Method_HSA							
Driller_	O.J. I	tarper		Log By MM KatzmanDate_5/2/9	4						
Depth (Feet)	Graphic Leg	Well Construction	Sample	Description/Soil Classif (Color, Texture, Struc	i						
				FILL (FILL)	Boring advanced with 4-1/4" HSA on truck						
_1				Low plastic, dark brown Silty CLAY.	CL) mounted Acker 82 rig.						
- 2				Low plastic, moist, tan Clayey SILT. (ML	.)						
_ 3											
- 4											
- 5				Becoming medium brown.							
- 6											
- 7											
- 8				Becoming damp.							
- g											
- 10											
- 11				Becoming very fine, Silty SAND to Sandy SILT.	(SM)						
- 12											
1 42											

Number MW14A

Depth (Feet)	phic	_ttou	Sample	Description/Soil Classification
	Graphic Log	Well	8 2	(Color, Texture, Structures)
13				SAME: Medium brown, moist, very fine grained Silty SAND with Sandy SILT. (SM)
14				
<u> </u>				
— 16				With mixed Sandy SILT and Clayey SILT. (SM w/ ML)
<u> </u>				
18				
"				Becoming wet and light medium brown.
- 19				
				With much fine SAND.
- 20				
<u>-21</u>				No return.
-21				
				Medium grained SAND. (SP)
23				
24				
25				SAME SILVERS
				Medium fine, light brown SAND with Silt. (SP)
<u> </u>				
				Free water.
<u> </u>				TIOC HUCG.
 28				Fine to medium, light brown SAND.
—29				
■ [_30_]	L			Page <u>2</u> of <u>3</u>

Number MW14A

Depth (Feet)	Graphic Log	Well	Sumple	Description/Soil (Color, Texture		-
38	\vdash	8	-	SAME: Medium , light brown SAND.		
31				Medium with same fine SAND with tr. of silt a	nd lignite.	
— 32				Very fine Silty SAND with Sandy SiLT.	(SM w/ ML)	
Je						Bottom of Boring 32 ft.
— 33						
- 34						
35						
36						
<u> </u>						
-38						
39						
40						
—41 J						
- 42						
— 43						
— 44						
— 45						
—46						
47_						

Project: Unison

Owner: Union Carbide Corp.

Location: Kansas City, KS W.O. Number: K41001.0

Number:

S-1

Total Depth: 60 ft. Diameter: 3-7/8" - 4"

Surface Elevation: 745 msl

Water Level: Init: N/A

24hrs: N/A

Screen: Dia: NA

Length: N/A

Slot Size: NA

Casing: Dia: N/A

Length: N/A Type: N/A

Drilling Company: Layne Western Drilling Method: Rotary Mud

Drilling Log Sketch Map ⊗ S-1 Bldg. Notes

Driller	۱.۲٥ :	larper	Log	By: MM Katzman Date: 4/20/94
Depth Feet)	very	very		Description/Soil Classification
	Recovery	SpT	Sample Number	(Color, Texture, Structures)
-0-				Gravel and clay-silt BACKFILL. Boring advanced with 3-7/8"
				fishtail-drag bit, rotary method and bentonite slurry
				Gardner Derwer Rig Samples obtained with 2"
- 2				Firm, dark brown, low plastic, Clayey SILT with tr. fine sand (ML) diameter split spoon
	13/24	5		
L 3		5	511	
		2		Becoming firm to base of silt
- 4		5		
	21/24	3	512	Firm to stiff dense, brown low plastic Silty CLAY with fine (CL) sand and some clayey silt with red brown mottling.
- 5		4		Sain and some clayey site with rea to own mortaling.
		6		
- 6		8	1	With Clayey SILT lenses (W/ML)
	15/24	2	S13	
-7		Э		
` <i>`</i>		2		Becoming firm and very thinly bedded to laminated with decreasing silt lenses. Clay is mottled red-brown. (CL)
- 8		2		
	23/24	2	514	
Lal		1		Very lease this hadded mattled grow and brown low plants Clavey Sil T. a. s.
9		2		Very loose, thin bedded mottled gray and brown, low plastic Clayey SILT (ML) and thin CLAY lenses with fine sand.
- 10		4		Approximately 30% DWL between 8 and 10 ft.
	24/24	2	S15	
_ 11		2		With sand lens at 11 ft.
		3		Becoming loose, thin bedded laminated gray and brown Clayey SILT. (ML)
		4		
12	24/24	4 4 6 9	S16	Medium dense thin bedded, gray and brown, very fine grained SAND and Silty SAND, poorly graded. (SM-SP)

Environmental Resources Management Project: Unison

Owner: Union Carbide Corp.

Location: Kansas City, KS W.O. Number: K41001.0

Number: S-1

Total Depth: 60 ft.

Diameter 3-7/8" - 4"

Surface Elevation: 745 msl

Water Level: Init: N/A 24 hrs N/A

Screen: Dia: N/A

Length: N/A

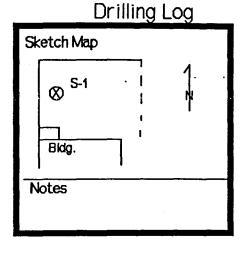
Slot Size: N/A

Casing: Dia: N/A

Length: N/A Type: N/A

Drilling Company: Layne Western Drilling Method: Rotary Mud

Log But MM Vataman



Driller:	٥٦.	Harper	_	Log By: MM Katzman Date: 4/20/94
Depth (Feet)	Recovery	SPT	Sample	Description/Soil Classification (Color, Texture, Structures)
	Ř	ا م	Semple	(Color, Texture, 3d dectures)
13				Same: medium dense thin bedded, gray and brown very fine SAND and Silty SAND - poorly graded. (SM-SP)
<u> 14 </u>		7	 	
	18/24	B	S17	
15		-		Medium dense, thin bedded/laminated gray brown low plastic Sandy SILT with thin
"		6		clay laminates. (SM)
	1	8		Made dance this hadded mixed years fine City CAND and Candy Cil T with fine cand
16	12/24	8	S18	Medium dense, thin bedded mixed very fine Silty SAND and Sandy SILT with fine sand. (SMw/SP
	ł	13		
- 17		15		
1 11		15		Medium dense, light gray to brown, very fine to medium grained SAND with tr. siilt - poorly graded - dry (SP)
F 18		13		poorly graded 'day'
"	14/24	12	519	with thin black lignite/organic layer at 19ft.
-	l	12		Becoming brown gray and fine to medium grained - saturated
- 19	1	12		
1 11	l	13		
20		13	S110	(continued water loss, approximately 30% DWL).
1 11	10/24	1	3110	
_ 21		3		Becoming loose and medium grained with tr. of fine sand. (SP)
		7		
		14		
22	DIAD		S111	Becoming dense and medium grained with thin lenses of brown sand and black lignite
1 11	9/18	19		streaks and tr. fine gravel.
23	ļ	20		
-		24	ļ	
- 24	6/18	13	S112	Becoming very dense and medium grained with some coarse sand and tr. fine gravel with light. (SP)
]]]		24		lignite. (SP)
- 25		1		
-		31		
				Becoming medium to fine SAND with tr. of coarse sand and some silt. (SP)

Project: Unison

Owner: Union Carbide Corp.

Location: Kansas City, KS

W.O. Number: K41001.0

Number: S-1

Total Depth: 60 ft. Diameter: 3-7/8" - 4"

Surface Elevation: 745 msl Water Level: Init: N/A

24 hrs: N/A

Drilling Log

Sketch Map

⊗^{S-1}

Bldg.

Notes

Screen: Dia: N/A

Length: N/A

Slot Size: N/A

Casing: Dia: N/A

Length: N/A

Type: N/A

Drilling Company: Layne Western Drilling Method: Rotary Mud

Drille	r: ۵៕	Harper		Log By: MM Katzman Date: 4/20/94
Depth (Feet)	Recovery	SPT	Sample	Description/Soil Classification (Color, Texture, Structures)
26	11/18	12 24	5113	Same: Very dense, medium to fine SAND with tr. of coarse sand and some slit, with fine lignite, poorly graded, brown with some gray. (SP)
- 27		27		
- 28	7/18	15	S114	Becoming fine grained with tr. medium sand and tr. of Silt with SILT lenses. (WISM)
- 29		13 10		Reamed with 5-7/8" fishtall, set 4" casing with bentonite seal to 30 ft.
30	5/18	4	5115	Loose, gray, very fine grained SAND, poorly graded to Silty SAND with tr. medium
- 31		5 4		sand and silt with coal/lignite bed at 31.5 ft. (SP-SM)
- 32	8/18	11	S116	Dense, dark gray, poorly graded and fine grained Silty SAND with lignite
22		15		and traces of medium sand. (SM)
– 33		23		
34	11/18	11	5117	Becoming fine grained.
35		24		Becoming dense - as above (SM)
		26		with clay gull at tip.
36	7/18	10	S118	Becoming medium dense, traces of medium sand. (SM)
- 37		13 20		
— эв				Medium dense with wood chips at top of sample and 1" lignite bed at base of sample.
	11/18	7 10 13	5119	Medicin dense with wood thips at top of sample and a lightle bed at base of sample.
ו סבו	\Box			

Project: Unison

Owner: Union Carbide Corp.

Location: Kansas City, KS W.O. Number: K41001.0

Number: 5-1

Total Depth: 60 ft. Diameter: 3-7/8" - 4"

Surface Elevation: 745msl

Water Level: Init: N/A 24 hrs: N/A

Screen: Dia: N/A Length: N/A

Slot Size: N/A

Casing: Dia: N/A

Length: N/A Type: N/A

Drilling Company: Layne Western Drilling Method: Rotary Mud

Drilling Log Sketch Map **⊗**S-1 Bldg. Notes

Drille		Harper	Lo	pg By: MM Katzman Date: 4/20/94
Depth (Feet)	Recovery	SPT	Sample Number	Description/Soil Classification (Color, Texture, Structures)
33				SAME: Medium dense with wood chips at top of sample and 1" lignite bed at base of sample.
40				Very dense, poorly graded Interbedded fine SAND and Silty SAND. (SM-SP)
41	11/18	9 24 30	S120	Layered with lignite.
- 42	12/17	16	S121	
- 43		41 50/5*		As above, with some medium grained SAND with tr. of powdered rock on tip.
				Dense, fine grained, gray, poorly graded SAND with some silt and tr. of coarse sand. (SP)
44	9/18	15	S122	
45		19 15		
- 46				
- 47	6/18	9	5123	Becoming medium dense.
		16		Medium dense, medium grained, poorly graded dark gray SAND with tr. fine sand and
- 4B				some coarse grained sand. (SP)
	3/18	10 29	5124	
49		36		Dense fine to medium grained, gray, poorly graded SAND with lenses of Silty SAND and thin (1") lens of fine gravel, with 2" thick lignite rich zone. (SP w/ SM)
- 50				
	12/18	19	5125	Dense, gray, very Silty SAND with traces of gravel and coarse sand, poorly graded thinly laminated/bedded. (SM)
_ 51		24 26		
52				

Project: Unison

Owner: Union Carbide Corp.

Location: Kansas City, KS W.O. Number: K41001.0

Number: S-1

Total Depth: 60 ft. Diameter: 3-7/8" - 4"

Surface Elevation: 745msl Water Level: Init: N/A 24 hrs: N/A

Length: N/A Screen: Dia: N/A Slot Size: N/A

Casing: Dia: N/A

Length: N/A

Type: N/A

Drilling Company: Layne Western Drilling Method: Rotary Mud

Drilling Log Sketch Map **⊗** S-1 Bldg. Notes

Drille	r: 0.J	. Harpe	r	Log By: MM Katzman Date: 4/20/94		
Depth (Feet)	Recovery	SPT	Semple Number	Description/Soil Classification (Color, Texture, Structures)		
-52 - 53	10/18	14 12	5126	SAME: Dense, gray, very Silty SAND with tr. of gravel and coarse sand, poorly graded, thinly laminated/bedded. (SM)		
- 54		11		Becoming medium dense with lignite gravel.		
55	18/18	3 6 7	5127	Interbedded, medium dense gray Clayey SILT with thin laminae/beds of stiff Silty CLAY with some Sandy SILT - with traces fine sand with thin (1/8") lignite lenses. (ML w/ OL & SM)		
- 56	8/18	7 18	S128	Dense, very fine grained Silty SAND, gray with tr. of medium sand, laminated with lignite. (SM)		
- 57 - 57		21		Dense, medium to coarse grained, poorly graded gray SAND with some fine sand. (SP)		
_ 58 59	12/24	13 14	5129	Becoming medium dense, fine grained SAND, gray, poorly graded. (SP)		
- 60		14 15		Medium dense, gray, coarse grained, poorly graded SAND with traces of medium grained sand with fine gravel in tip. (SP)		
-				Bottom of boring 60 ft. 4/20/94. Boring grouted 4/21/94.		
				·		

APPENDIX C



Unison Transformer Services 126 Brinkerhoff Road Hansas City, KS 65115

May 16, 1994

PACE Project Number: 54051051

ttn: Mr. Mark Liggatt

Client Reference: Well Sampling

CE Sample Number: Lete Collected: Date Received:

60 0067996 05/10/94 05/10/94

			05/10/94		
<u>Frameter</u>	Units	MDL	PZ 1A		
ORGANIC ANALYSIS	0.1703	_ PIUL		METHOD	DATE ANALYZED
ANALISIS					
VEATILE ORGANICS					
Wethyl chloride (Chloromethane)				8240	
dethyl bromide (Bromomethane)	ug/L	200	ND		05/13/94
Vayl Chloride	ug/L	200	ND		05/13/94
oroethane	ug/L	200	ND		05/13/94
lethylene Chloride	ug/L	200	ND		05/13/94
(cetone	ug/L	100	ND		05/13/94
	ug/L	200	ND		05/13/94
ambon Disulfide					00/10/24
,1-Dichloroethylene	ug/L	100	ND		05/13/94
Dichloroethane	ug/L	100	ND		05/13/94
Dichloroethylene (Total)	ug/L	100	ND		05/13/94
11:01:01:01:01:01	ug/L	100	2100		05/13/94
.2-Dichloroethane	ug/L	100	ND		05/13/94
•	ug/L	100	ND		05/13/94
- tanone (MEK)		1.00			
,l,l-Trichloroethane	ug/L	100	ND		05/13/94
arbon Tetrachloride	ug/L	100	ND		05/13/94
Acetate	ug/L	100	ND		05/13/94
orobromomethane	ug/L ug/L	100	ND		05/13/94
2-Dichloropropane	ug/L	100	ND		05/13/94
	ug/ c	100	ND		05/13/94
1,3-Dichloropropene	ug/L	100	No		•
'ETIIOTOEThylene	ug/L	100	ND		05/13/94
lorodibromomethane		100	2400		05/13/94
lm2-Trichloroethane	ug/L ug/L	100	ND		05/13/94
nene	ug/L ug/L	100	ND		05/13/94
ans-1,3-Dichloropropene	ug/L	100	ND		05/13/94
· · · · · · · · · · · · · · · · · · ·	ug/ L	100	ND		05/13/94
oform	ug/L	100	ND		•
∰hyl-2-Pentanone (MIBK)	ug/L	100	ND ND		05/13/94
rexamone	ug/L	100	ND ND		05/13/94
rachloroethylene	ug/L	100	ND ND		05/13/94
,2-Tetrachloroethane	ug/L	100	ND		05/13/94
	~		III		05/13/94



. Mark Liggatt 2

May 16, 1994 PACE Project Number: 54051051

Client Reference: Well Sampling

ACE Sample Number: Date Collected:

60 0057996 05/10/94

Date Collected: Date Received: I fent Sample ID: Parameter	Unit <u>s</u>	MOL	05/10/94 05/10/94 PZ 1A	METUOD	DATE ANALYZED
	011163	<u> </u>		HETHOD	DATE ANALIZED
GANIC ANALYSIS					
VOLATILE ORGANICS				8240	
Le l yene	ug/L	100	ND		05/13/94
lorobenzene	ug/L	100	ND		05/13/94
Ethylbenzene	ug/L	100 100	ND ND		05/13/94
Styrene Flenes (Total)	ug/L ug/L	100	ND D		05/13/94 05/13/94
chlorodifluoromethane	ug/L	200	ND		05/13/94
	-3/ -		•••		
Trichlorofluoromethane	ug/L	100	ND		05/13/94
rolein	ug/L	2000	ND		05/13/94
Acrylonitrile Iodomethane	ug/L	2000	ND ND		05/13/94
bromomethane	ug/L ug/L	1 00 100	ND ND		05/13/94 05/13/94
Chloroethylvinyl ether (mixed)	ug/L	100	ND		05/13/94
Shylmethacrylate	ug/L	100	ND		05/13/94
2,3-Trichloropropane 1,4-Dichloro-2-butene	ug/L	100 100	ND ND		05/13/94 05/13/94
1.3-Dichlorobenzene	ug/L ug/L	100	ND		05/13/94
4-Dichlorobenzene	ug/L	100	ND		05/13/94
2-Dichlorobenzene	ug/L	100	ND		05/13/94
2-Dichloroethane-d4 (Surrogate)	%		99		05/13/94
luene-d8 (Surrogate)	%		104	•	05/13/94
4-Bromofluorobenzene (Surrogate)	%		103		05/13/94
STICIDES/PCBS				8080/608	3
Amoclor 1016	ug/L	1.0	ND	•	05/13/94
Aroclor 1221	ug/L	2.0	ND		05/13/94
Apoclor 1232	ug/L	1.0	ND		05/13/94
Apolor 1242 Aroclor 1248	ug/L	1.0 1.0	ND ND		05/13/94 05/13/94
Aroclor 1254	ug/L ug/L	1.0	ND		05/13/94
	~ ∃/ ←		***		44, 44, 4
Amoclor 1260	ug/L	1.0	ND		05/13/94
Decachlorobiphenyl Surrogate	%		125		05/13/94
Ttrachloro-meta-xylene Surrogate	· %		88		05/13/94

P. 03



REPORT OF LABORATORY ANALYSIS

Mark Liggatt age

May 16, 1994 PACE Project Number: 54051051

8080/608

#152 P04

Client Reference: Well Sampling

ACE Sample Number: Date Collected:

60 0067996 05/10/94

Date Received: ient Sample ID: <u>arameter</u>

05/10/94 PZ 1A

Units

MDL

METHOD DATE ANALYZED

ARGANIC ANALYSIS

sticides Prep

PESTICIDES/PCBS Dibutylchlorendate (Surrogate)

63 05/11/94

05/13/94

9608 Loiret Soulevard Lenexa, KS 66219 TEL: 913-569-6668 FAX: 913-599-1759

An Equal Opportunity Employer



60 0068003

ir Mark Liggatt

May 16, 1994

PACE Project Number: 540510512

lient Reference: Well Sampling

YEE Sample Number: late Collected:

late Collected:

ent Sample ID:

05/10/94 ed: 05/10/94 le ID: PZ 2A

:Tent Sample ID:			YL ZA		•
ameter	<u>Units</u>	MDL	· · · · · · · · · · · · · · · · · · ·	<u>METHOD</u>	DATE ANALYZED
					
PREANIC ANALYSIS					
MATTER ODCANICS				8240	
ICEATILE ORGANICS Lethyl chloride (Chloromethane)	ug/L	5000	ND		05/13/94
tethyl chipride (Chiprimethane)	ug/L	5000	ND		05/13/94
/i yl Chioride	ug/L	5000	ND		05/13/94
Chloroethane	ug/L	5000	ND		05/13/94
leshylene Chloride	ug/L	2500	ND		05/13/94
to tone	ug/L	5000	ND		05/13/94
•			***		05 (12 (04
Carbon Disulfide	ug/L	2500	ND		05/13/94
l ≝ -Dichloroethylene	ug/L	2500	ND		05/13/94
lDichloroethane	ug/L	2500	ND		05/13/94
1,2-Dichloroethylene (Total)	ug/L	2500	ND ND		05/13/94 05/13/94
hloroform	ug/L	2500 2500	ND ND		05/13/94
lDichloroethane	ug/L	2500	ND		03/13/34
 ?-Butanone (MEK)	ug/L	2500	ND		05/13/94
1 2 ,1-Trichloroethane	ug/L	2500	ND		05/13/94
bon Tetrachloride	ug/L	2500	ND		05/13/94
/inyl Acetate	ug/L	2500	ND		05/13/94
)ich]orobromomethane	ug/L	2500	ND		05/13/94
i Dichloropropane	ug/L	2500	ND		05/13/94
	4.	2500	NO		OE /12 /04
Cis-1,3-Dichloropropene	ug/L	2500	ND OF OCC		05/13/94 05/13/94
Inchloroethylene	ug/L	2500	25000 ND		05/13/94
: Imorodibromomethane	ug/L	2500 2500	ND ND		05/13/94
1,1,2-Trichloroethane	ug/L	2500 2500	ND		05/13/94
3enzene	ug/L	2500 2500	ND ND		05/13/94
funs-1,3-Dichloropropene	ug/L	2500	ND		00/10/51
3romoform	ug/L	2500	ND		05/13/94
1 third 107 iii	ug/L	2500	ND		05/13/94
: examine	ug/L	2500	ND		05/13/94
[etrachloroethylene	ug/L	2500	ND		05/13/94
1_,2,2-Tetrachloroethane	ug/L	2500	ND		05/13/94
(uene	ug/L	2500	ND		05/13/94
•	·	ብሮለላ	MD		05/13/94
Chlorobenzene	ug/L	2500	ND		03/13/34



r. Mark Liggatt

May 16, 1994 PACE Project Number: 54051051

ient Reference:	Well	Sampling
-----------------	------	----------

60	0068003	
05/	10/94	
	10/94	
D7	24	

PACE Sample Number: Date Collected: Lete Received: Lent Sample ID:			60 0068003 05/10/94 05/10/94 PZ 2A		
<u>Parameter</u>	<u>Units</u>	MDL		METHOD	DATE ANALYZED
GANIC ANALYSIS					
VOLATILE ORGANICS				8240	
hylbenzene	ug/L	2500	ND		05/13/94
Tyrene (Tetal)	ug/L	2500	ND		05/13/94
Xylenes (Total) Dichlorodifluoromethane	ug/L	2500 5000	ND ND	+	05/13/94
ichlorofluoromethane	ug/L ug/L	2500	ND		05/13/94 05/13/94
Acrolein	ug/L	500 00	ND		05/13/94
Minus and had a					• •
/ rylonitrile domethane	ug/L	50000	ND		05/13/94
Dibromomethane	ug/L	2500	ND ND		05/13/94
2_Chloroethylvinyl ether (mixed)	ug/L	2500 2500	ND ND		05/13/94
Enylmethacrylate	ug/L ug/L	2500	ND		05/13/94 05/13/94
1,2,3-Trichloropropane	ug/L	2500	ND		05/13/94
The Bulletin of the	•				•
124-Dichloro-2-butene	ug/L	2500	ND:		05/13/94
18-Dichlorobenzene	ug/L	2500	ND		05/13/94
1,4-Dichlorobenzene 1_2-Dichlorobenzene	ug/L	2500	ND		05/13/94
22-Dichloroethane-d4 (Surrogate)	ug/L	2500	ND		05/13/94
Teluene-d8 (Surrogate)	% %		102 102		05/13/94 05/13/04
reruene-do (surroyave)	*		102		05/13/94
#Bromofluorobenzene (Surrogate)	%		102		05/13/94
PESTICIDES/PCBS				8080/608	
Aroclor 1016	ug/L	1.0	ND		05/13/94
Apolor 1221	ug/L	2.0	ND		05/13/94
Amoclor 1232 Aroclor 1242	ug/L	1.0	ND		05/13/94
Ampolor 1248	ug/L	1.0	ND ND		05/13/94
Apclor 1254	ug/L	1.0	ND ND		05/13/94
A COUNTY OF THE PROPERTY OF TH	ug/L	1.0	NU		05/13/94
Aroclor 1260	ug/L	1.0	ND		05/13/94
Decachlorobiphenyl Surrogate	%		143		05/13/94
Tarachloro-meta-xylene Surrogate	%		90		05/13/94
Dibutylchlorendate (Surrogate)	%		87		05/13/ 9 4
Posticides Prep			05/11/94		

FAX NO. 9135991759



REPORT OF LABORATORY ANALYSIS

Mark Liggatt

May 16, 1994

PACE Project Number: 540510512

lient Reference: Well Sampling

ACE Sample Number: ate Collected: Received:

60 0068011 05/10/94

ent Sample ID: arameter

05/10/94 PZ 3A MDL METHOD DATE ANALYZED Units

RINIC ANALYSIS					
OLATILE ORGANICS				8240	
emyl chloride (Chloromethane)	ug/L	500	ND	V2 1 V	05/13/94
anyl browide (Brownethane)	ug/L	500	ND		05/13/94
iny) Chloride	ug/L	500	ND	•	05/13/94
hleroethane	ug/L	500	ND		05/13/94
	ug/L	250	ND		05/13/94
emylene Chloride	ug/L	500	ND		05/13/94
reroue	ug/ c	•••			
amon Disulfide	ug/L	250	ND	·	05/13/94
, Dichloroethylene	ug/L	250	ND		05/13/94
,1-Dichloroethane	ug/L	250	ND		05/13/94
,2-Dichloroethylene (Total)	ug/L	250	290		05/13/94
1 roform	ug/L	250	NO		05/13/94
. Dichloroethane	ug/L	250	ND		05/13/94
,2 0,011,010000000	J ,				
- latanone (MEK)	ug/L	250	NĎ		05/13/94
. 1-Trichloroethane	ug/L	250	ND		05/13/94
arbon Tetrachloride	ug/L	250	ND		05/13/94
inyl Acetate	ug/L	250	ND		05/13/94
orobromomethane	ug/L	250	ND		05/13/94
Dichloropropane	ug/L	250	ND		05/13/94
E-Diction of object	~5 / ~				• •
. 3-Dichloropropene	ug/L	250	ND		05/13/94
inloroethylene	ug/L	250	4100		05/13/94
Torodibromomethane	ug/L	250	ND		05/13/94
1.2-Trichloroethane	ug/L	250	ND		05/13/94
indene	ug/L	250	ND -		05/13/94
'ams-1,3-Dichloropropena	ug/L	250	ND		05/13/94
	•				o# 43.0 /0.1
omoform	ug/L	250	ND		05/13/94
Mathyl-2-Pentanone (MIBK)	ug/L	250	ND		05/13/94
Hexanone	ug/L	250	ND		05/13/94
trachloroethylene	ug/L	250	ND		05/13/94
12,2-Tetrachloroethane	ug/L	250	ND		05/13/94
lene	ug/L	250	ND		05/13/94
	-				AR /3 A /A /
lerobenzene	ug/L	250	ND		05/13/94
	•				

PACE, INC

FAX NO. 9135991759

P. 08



REPORT OF LABORATORY ANALYSIS

Mark Liggatt 7

May 15, 1994 PACE Project Number: 540510512

Geient Reference:	: Well	Sampling
--------------------------	--------	----------

PACE Sample Number:	
Date Collected:	
Te Received:	
Crient Sample ID:	

PACE Sample Number: Date Collected: Leta Received: Crient Sample ID: Parameter CAGANIC ANALYSIS	<u>Units</u>	MDL	60 0068011 05/10/94 05/10/94 PZ 3A		ATE ANALYZED
VOLATILE ORGANICS Employene Syrene Xylenes (Total) Cichlorodifluoromethane Inthlorofluoromethane Acrolein	ug/L ug/L ug/L ug/L ug/L ug/L	250 250 250 500 250 500	ND ND ND ND ND	8240	05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
Arylonitrile Indomethane Dibromomethane Chloroethylvinyl ether (mixed) Enylmethacrylate 1,2,3-Trichloropropane	ug/L ug/L ug/L ug/L ug/L ug/L	5000 250 250 250 250 250	ND ND ND ND ND ND		05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
154-Dichloro-2-butene 153-Dichlorobenzene 1,4-Dichlorobenzene 1,2-Dichlorobenzene 152-Dichloroethane-d4 (Surrogate) 151uene-d8 (Surrogate)	ug/L ug/L ug/L ug/L %	250 2 50 250 250	ND ND ND ND 101 102		05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
PESTICIDES/PCBS Arpclor 1016 Apclor 1221 Arbclor 1232 Aroclor 1242 Apclor 1248 Apclor 1254	% ug/L ug/L ug/L ug/L ug/L ug/L	1.0 2.0 1.0 1.0 1.0	ND ND ND ND ND ND ND	8080/608	05/13/94 05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
Aroclor 1260 Cachlorobiphenyl Surrogate Tetrachloro-meta-xylene Surrogate Dibutylchlorendate (Surrogate) Pasticides Prep	ug/L % % %	1.0	ND 125 94 68 05/11/94		05/13/94 05/13/94 05/13/94 05/13/94

PACE, INC

FAX NO. 9135991759

P. 07



REPORT OF LABORATORY ANALYSIS

. Mark Liggatt

May 16, 1994

PACE Project Number: 54051051:

ient	Reference:	Mell	Sampling	

PACE Sample Number:

60 0068020

Date Collected: Lite Received: Liter Sample ID:			05/10/94 05/10/94 PZ 4A	
Parameter	Units	MDL	12 77	METHOD DATE ANALYZED
GANIC ANALYSIS				
VOLATILE ORGANICS				8240
thyl chloride (Chloromethane)	ug/L	10	ND	05/12/94
thyl bromide (Bromomethane)	ug/L	10	ND	05/12/94
Vinyl Chloride	ug/L	10	ND	05/12/94
★loroethane	ug/L	10	ND	05/12/94
thylene Chloride	ug/L	5	ND	05/12/94
Acetona	ug/L	10	ND	05/12/94
■rbon Disulfide	ug/L	5	ND	05/12/94
1-Dichloroethylene	ug/L	5 5 5 5 5 5 5	ND	05/12/94
I,1-Dichloroethane	ug/L	5	ND	05/12/94
12-Dichloroethylene (Total)	ug/L	5	13	05/12/94
loroform	ug/L	5	ND:	05/12/94
17 2-Dichloroethane	ug/L	5	ND	05/12/94
Butanone (MEK)	ug/L	5	ND	05/12/94
1,1-Trichloroethane	ug/L	5	ND	05/12/94
Carbon Tetrachloride	ug/L	5 5 5 5 5 5	ND	05/12/94
Vinyl Acetate	ug/L	5	ND	05/12/94
chlorobromomethane	ug/L	5	ND	05/12/94
72-Dichloropropane	ug/L	5	ND	05/12/94
ms-1,3-Dichloropropené	ug/L	5	ND	05/12/94
ichloroethylene	ug/L	5 5 5 5 5 5	88	05/12/94
Chlorodibromomethane	ug/L	5	ND	05/12/94
1.1.2-Trichloroethane	ug/L	5	ND	05/12/94
nzene	ug/L	5	ND	05/12/94
ans-1,3-Dichloropropene	ug/L	5	ND	05/12/94
Emomo form	ug/L	5	ND	05/12/94
		Ĕ	ND	05/12/94
Methyl-2-Pentanone (MIBK) 2-Hexanone	ug/L	S K	иD	05/12/94
Z-nexamone Tetrachloroethylene	ug/L ug/L	ຍ ຮ	ND	05/12/94
1,2,2-Tetrachloroethane	ug/L	5	ND	05/12/94
Thuene	ug/L	5 5 5 5 5	ND	05/12/94
	••		_	

ug/L

lorobenzene

ND

05/12/94

#152 PØ9

P. 08

INCORPORATED THE ASSURANCE OF QUALITY

REPORT OF LABORATORY ANALYSIS

Mark Liggatt

May 16, 1994 PACE Project Number: 540510512

8240

: Reference:	Well	Sampling
--------------	------	----------

ACE Sample Number:)ate Collected: e Received: Trent Sample ID:

60 0068020 05/10/94 05/10/94 PZ 4A

ND

ND ND ND ND ND

ND

ND ND ND ND ND

ND ND ND ND 96

100

100

ND

ND ND ND ND ND

05/11/94

44.4	F	ANIC	ANALYSIS
------	---	------	----------

(OLATILE ORGANICS

- vibenzene

'arameter

Strene	
Kylenes (Total)	
Aylenes (local)	
hlorodifluorometh	MILE
I chlorofluorometha	ine
Acrolein	

Advionitrile Ledomethane Dibromomethane 2 thloroethylvinyl Edylmethacrylate 1,2,3-Trichloropro	(mixed)

1 -Dichlore	p- 2-butene	
1 -Dichlor	obenz e ne	
1.4-Dichlor	obenzene	
1-2-Dichlor	obenz e ne	
1 Dichlor	oethane-d4	(Surrogate)
Toluene-d8	(Surrogate)	

PESTICIDES/PCBS

Bromofluorobenzene	(Surrogate)
--------------------	-------------

. 20 1 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
Amoclor 1016
Apolor 1221
Aroclor 1232
Aroctor 1242
Moclor 1248
Actor 1254
/#BUC101 1294
Amoclor 1260
Wactor, 1500
cachlorobiphenyl Surrogate
tetrachloro-meta-xylene Surrogate
Lettaculoto-mera-xalana antioanne
Dibutylchlorendate (Surrogate)
Olding the transfer of the second
sticides Prep
and the state of t

ug/L	5
ug/L	5
ug/L	5
ug/L	10
1.00	Ę.

Units

MDL

ug/L ug/L	100
ug/L	100
ug/L	5

ug/L ug/L %	5 5
%	

ug/L

ug/L	2.0
ug/L	1.0
uy/ L	

1.0

ug/L %	1.0
%	
%	

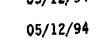
05/12/94
05/12/94
03/12/37
05/12/94
05/12/94

05/12/94 05/12/94 05/12/94	-
05/12/94	

05/12/94
05/12/94
05/12/94
05/12/94
05/12/94

05 (10 (04
05/12/94 05/12/94
05/12/94

05/	12/94
05/	12/94
	12/94
05/	12/94



	4 4 / 1
080/608	
•	05/13/94

8080/608	A= (10 /01
	05/13/94 05/13/94
	05/13/94
	05/13/94
	05/13/94

05/13/94 05/13/94	
OE /13/94	

05/13/94	ł
05/13/94	
05/13/94	ļ
05/13/94	1

PACE, INC

FAX NO. 9135991759

P. 09



REPORT OF LABORATORY ANALYSIS

Na. Mark Liggatt Nage 10 May 16, 1994

PACE Project Number: 540510512

Gaient Reference: Wall Sampling

PACE Sample Number: Date Collected: 60 0068038 05/10/94

te Received: lent Sample ID: 05/10/94 MW 11A

<u>Parameter</u>

Units MDL METHOD DATE ANALYZED

CEANIC ANALYSIS				
VOLATILE ORGANICS			8240	
Hethyl chloride (Chloromethane)	ug/L	20000	ND	05/14/94
Mathyl bromide (Bromomethane)	ug/L	20000	ND .	05/14/94
Vinyl Chloride	ug/L	20000	ND	05/14/94
Chiloroethane	ug/L	20000	ND	05/14/94
Hethylene Chloride	ug/L	10000	ND	05/14/94
Acetone	ug/L	20000	ND	05/14/94
VCETOHA	ug/ L	2000	NO	••/ • // • ·
Carbon Disulfide	ug/L	10000	ND	05/14/94
1 ■1-Dichloroethylene	ug/L	10000	ND	05/14/94
1,1-Dichloroethane	ug/L	10000	ND	05/14/94
1_2-Dichloroethylene (Total)	ug/L	10000	120000	05/14/94
Coroform	ug/L	10000	ND '	05/14/94
1,2-Dichloroethane	ug/L	10000	ND	05/14/94
-,2 5.0	gr	- -		• •
2 Butanone (MEK)	ug/L	10000	ND	05/14/94
11,1-Trichloroethane	ug/L	10000	ND	05/14/94
Carbon Tetrachloride	ug/L	10000	ND	05/14/94
	ug/L	10000	ND	05/14/94
Vinyl Acetate Dichlorobromomethane	ug/L	10000	ND	05/14/94
1,2-Dichloropropane	ug/L	10000	ND	05/14/94
2 f 2 - 0 f cit f of optional c	W3/ C	10000		
C=:-1,3-Dichloropropene	ug/L	10000	ND	05/14/94
Thichloroethylene	ug/L	10000	360000	05/14/94
Chlorodibromomethane	ug/L	10000	ND 1	05/14/94
1.1.2-Trichloroethane	ug/L	10000	ND	05/14/94
1.1,2-Trichloroethane Banzene	ug/L	10000	ND	05/14/94
Teans-1,3-Dichloropropene	ug/L	10000	ND	05/14/94
The state of the s	4.5/			
Burnoform	ug/L	10000	ND	05/14/94
4 Methyl-2-Pentanone (MIBK)	ug/L	10000	ND	05/14/94
2-Hexanone	ug/L	10000	ND	05/14/94
Tetrachloroethylene	ug/L	10000	ND	05/14/94
1 , 2, 2-Tetrachloroethane	ug/L	10000	ND	05/14/94
luene	ug/L	10000	ND	05/14/94
	·· • · ·			
Chi orobenzene	ug/L	10000	ND	05/14/94
	-			

#152 P11

P. 10



REPORT OF LABORATORY ANALYSIS

Mark Liggatt e 11

May 16, 1994

PACE Project Number: 540510512

Client Reference: Well Sampling

E Sample Number: Date Collected:

60 0068038 05/10/94

05/10/94

e Received:

Clent Sample 10: Parameter	Units	MDL	MW 11A	METHOD	DATE ANALYZED
HANIC ANALYSIS					
OLATILE ORGANICS				8240	
thy I benzene	ug/L	10000	ND		05/14/94
Starene	ug/L	10000	ND		05/14/94
(yrenes (Total)	ug/L	10000	ND		05/14/94
Dichlorodifluoromethane	ug/L	20000	ND		05/14/94
frachlorofluoromethane	ug/L	10000	ND		05/14/94
Modern	ug/L	200000	ND		05/14/94
icrylonitrile	ug/L	200000	ND		05/14/94
depomethane	ug/L	10000	ND		05/14/94
)i b romomethane	ug/L	10000	ND		05/14/94
:-Chloroethylvinyl ether (mixed)	ug/L	10000	ND		05/14/94
it	ug/L	10000	ND		05/14/94
, 3-Trichloropropane	ug/L	10000	ND		05/14/94
.,4-Dichloro-2-butene	ug/L	10000	ND		05/14/94
, Dichlorobenzene	ug/L	10000	ND		05/14/94
-, T -Dichlorobenzene	ug/L	10000	ND		05/14/94
,Z-Dichlorobenzene	ug/L	10000	ND		05/14/94
. Dichloroethane-d4 (Surrogate)	%		106		05/14/94
omene-d8 (Surrogate)	%	•	103		05/14/94
-Bromofluorobenzene (Surrogate)	%		101		05/14/94
ESTICIDES/PCBS				8080/608	•
roclor 1016	ug/L	1.0	ND		05/13/94
rector 1221	ug/L	2.0	ND		05/13/94
relar 1232	ug/L	1.0	ND		05/13/94
roclor 1242	ug/L	1.0	1.06		05/13/94
racior 1248	ug/L	1.0	ND		05/13/94
relor 1254	ug/L	1.0	ND		05/13/94
roclor 1260	ug/L	1.0	3.87		05/13/94
emchlorobiphenyl Surrogate	%	-	190 (1)		05/13/94
e achloro-meta-xylene Surrogate	%		66		05/13/94
ibutylchlorendate (Surrogate)	%		56		05/13/94
e <u>st</u> icides Prep			05/13/94		- •
			• •		



Mark Liggatt ge 12

May 16, 1994

PACE Project Number: 54051051:

Client Reference: Well Sampling

PACE Sample Number:

Date Collected:

te Received:

60 0068046

05/10/94 05/10/94

(M ient Sample ID: <u>Parameter</u>	Units	MDL	MW 12A	METHOD	DATE ANALYZED
C GANIC ANALYSIS					
VOLATILE ORGANICS				8240	
Mathyl chloride (Chloromethane)	ug/L	10000	ND		05/14/94
Hethyl bromide (Bromomethane)	ug/L	10000	ND		05/14/94
Vinyl Chloride	ug/L	10000	ND		05/14/94
Chloroethane	ug/L	10000	ND		05/14/94
Mathylene Chloride	ug/L	500 0	ND		05/14/94
Acetone	ug/L	10000	ND		05/14/94
Carbon Disulfide	ug/L	5000	ND		05/14/94
1 Dichloroethylene	ug/L	50 00	ND		05/14/94
1,1-Dichloroethane	ug/L	500 0	ND		05/14/94
1_2-Dichloroethylene (Total)	ug/L	50 00	69000		05/14/94
Coloroform	ug/L	5000	ND		05/14/94
1 72-Dichloroe thane	ug/L	5000	ND		05/14/94
2mButanone (MEK)	ug/L	5000	ND		05/14/94
1 ,1-Trichloroethane	ug/L	5000	ND		05/14/94
Carbon Tetrachloride	ug/L	5000	ND		05/14/94
V <u>i</u> nyl Acetate	ug/L	5000	ND		05/14/94
Dechlorobromomethane	ug/L	5000	ND		05/14/94
1 D-Dichloropropane	ug/L	5000	ND		05/14/94
Cis-1,3-Dichloropropene	ug/L	5000	ND		05/14/94
Thichloroethylene	ug/L	5000	9 6000		05/14/94
Chlorodibromomethane	ug/L	5000	ND		05/14/94
1,1,2-Trichloroethane	ug/L	5000	ND		05/14/94
Benzene	ug/L	5000	ND		05/14/94
Tems-1,3-Dichloropropene	ug/L	500 0	ND		05/14/94
Binomoform	ug/L	5000	ND		05/14/94
4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	ug/L	500 0	ND		05/14/94
2 Hexanone	ug/L	5000	ND		05/14/94
Tetrachloroethylene	ug/L	5000	ND		05/14/94
1 ,2,2-Tetrachloroethane	ug/L	5000	ND		05/14/94
Teruene	ug/L	5000	ND		05/14/94
Chlorobenzene	ug/L	500 0	ND		05/14/94



Mark Liggatt re 13

-17-84 TUE 06:41

May 16, 1994

PACE Project Number: 540510512

Client Reference: Well Sampling

Par Sample Number: Date Collected:

60 0068046 05/10/94

05/10/94

e Received:

Clent Sample ID:			MW 12A		
Parameter	<u>Units</u>	MDL		<u>METHOD</u>	DATE ANALYZED
A ANIC ANALYSIS					
OLATILE ORGANICS				8240	
thylbenzene	ug/L	5000	ND		05/14/94
Marene Grenes (Total)	ug/L	5000	ND		05/14/94
li <u>c</u> hlorodifluoromethane	ug/L	5000	ND		05/14/94
rechlorofluoromethane	ug/L ug/L	10000 5000	ND ND		05/14/94
Colein	ug/L ug/L	100000	ND ND		05/14/94
	ug/ L	100000	מט		05/14/94
convloritrile	ug/L	100000	ND		05/14/94
opmethane	ug/L	5000	ND		05/14/94
intromomethane	ug/L	5000	ND		05/14/94
:-Chloroethylvinyl ether (mixed) the limethacrylate	ug/L	5000	ND		05/14/94
. 3-Trichloropropane	ug/L	5000	ND		05/14/94
teno- with or obt obetie	ug/L	5000	ND		05/14/94
, ← Dichloro-2-butene	ug/L	5000	ND		05/14/94
, Dichlorobenzene	ug/L	5000	ND		05/14/94
, Dichlorobenzene	ug/L	5000	ND		05/14/94
,2-Dichlorobenzene	ug/L	5000	ND		05/14/94
<pre>. Dichloroethane-d4 (Surrogate) obene-d8 (Surrogate)</pre>	%		101		05/14/94
omena-do (adirodate)	%		101		05/14/94
-Bromofluorobenzene (Surrogate)	%		9 9		05/14/94
ESTICIDES/PCBS				8080/608	
roclor 1016	ug/L	1.0	ND	••••	05/13/94
rd lor 1221	ug/L	2.0	ND		05/13/94
rd lor 1232	ug/L	1.0	ND		05/13/94
roclor 1242	ug/L	1.0	ND		05/13/94
roslor 1248	ug/L	1.0	ND		05/13/94
relor 1254	ug/L	1.0	ND		05/13/94
octor 1260	ug/L	1.0	ND		05/13/94
chlorobiphenyl Surrogate	%		170 (1)		05/13/94
achloro-meta-xylene Surrogate	%		79		05/13/94
butylchlorendate (Surrogate)	%		8 6		05/13/94
esticides Prep			05/11/94		• •





Mark Liggatt 14

May 16, 1994 PACE Project Number: 54051051

🗀 ient Reference: Well Sampling

ACE Sample Number: Date Collected: lte Received:

60 0068054 05/10/94

05/10/94 MW 12B ient Sample ID: **Parameter** Units MDL METHOD DATE ANALYZED GANIC ANALYSIS **VOLATILE ORGANICS** 8240 |thyl chloride (Chloromethane) 2000 ND uq/L 05/13/94 thyl bromide (Bromomethane) ug/L 2000 ND 05/13/94 VInyl Chloride ug/L 2500 ND 05/13/94 Chloroethane 2500 ND ug/L 05/13/94 thylene Chloride ug/L 1200 ND 05/13/94 etone ug/L 2500 ND 05/13/94 rbon Disulfide 1200 ND ug/L 05/13/94 1-Dichloroethylene 1200 ND ug/L 05/13/94 **1,**1-Dichloroethane ug/L 1200 ND 05/13/94 2-Dichloroethylene (Total) 1200 8700 ug/L 05/13/94 (loroform 1200 ug/L ND 05/13/94 2-Dichloroethane 1200 ug/L ND 05/13/94 **B**utanone (MEK) ug/L 1200 ND 05/13/94 .1-TrichToroethane 1200 ND ug/L 05/13/94 Carbon Tetrachloride 1200 ND ug/L 05/13/94 Vinyl Acetate ND ug/L 1200 05/13/94 chlorobromomethane 1200 ND ug/L 05/13/94 2-Dichloropropane ug/L 1200 ND 05/13/94 Cis-1,3-Dichloropropene ug/L 1200 ND 05/13/94 ichloroethylene 1200 17000 ug/L 05/13/94 Control or or of the control of the c 1200 ug/L ND 05/13/94 1,1,2-Trichloroethane ug/L 1200 ND 05/13/94 zene ug/L 1200 ND 05/13/94 ns-I,3-Dichloropropene 1200 ND ug/L 05/13/94 Bromoform ug/L 1200 ND 05/13/94 Methyl-2-Pentanone (MIBK) 1200 ND ug/L 05/13/94 Rexanone ND uq/L 1200 05/13/94 Tetrachloroethylene ND 05/13/94 ug/L 1200 1,2,2-Tetrachloroethane ug/L 1200 ND 05/13/94 uene ug/L 1200 ND 05/13/94 (blorobenzene ug/L 1200 ND 05/13/94

P. 14



REPORT OF LABORATORY ANALYSIS

lr. Mark Liggatt age 15

May 16, 1994 PACE Project Number: 54051051

Client Reference: Well Sampling

PACE Sample Number:
Date Collected:

60 0068054 05/10/94

Pate Received: lient Sample ID: rarameter	Units	MDL	05/10/94 MW 12B	METHOD	DATE ANALYZED
RGANIC ANALYSIS					
VOLATILE ORGANICS				8240	
£thy1benzene	ug/L	1200	ND	02.10	05/13/94
tyrene	ug/L	1200	ND		05/13/94
Tylenes (Total)	ug/L	1200	ND		05/13/94
<u>Dichlorodifluoromethane</u>	ug/L	2500	ND		05/13/94
richlorofluoromethane	ug/L	1200	ND		05/13/94
crolein	ug/L	25000	ND		05/13/94
Acrylonitrile	ug/L	25000	ND		05/13/94
pdomethane	ug/L	1200	ND		05/13/94
#ibromomethane	ug/L	1200	ND		05/13/94
2-Chloroethylvinyl ether (mixed)	ug/L	1200	ND .		05/13/94
thylmethacrylate	ug/L	1200	ND		05/13/94
2,3-Trichloropropane	ug/L	1200	ND		05/13/94
4-Dichloro-2-butene	ug/L	1200	ND		05/13/94
3-Dichlorobenzene	ug/L	1200	ND		05/13/94
	ug/L	1200	ND		05/13/94
1,2-Dichlorobenzene	ug/L	1200	ND		05/13/94
2-Dichloroethane-d4 (Surrogate)	*		93		05/13/94
luene-d8 (Surrogate)	%		97		05/13/94
4-Bromofluorobenzene (Surrogate)	%		93		05/13/94
ESTICIDES/PCBS				8080/608	
Aroclor 1016	ug/L	1.0	ND	,	05/13/94
moclor 1221	ug/L	2.0	ND		05/13/94
coclor 1232	ug/L	1.0	ND		05/13/94
Aroclor 1242	ug/L	1.0	ND		05/13/94
Aroclor 1248	ug/L	1.0	ND		05/13/94
oclor 1254	ug/L	1.0	ND		05/13/94
Aroclor 1260	ug/L	1.0	ND		05/13/94
mcachlorobiphenyl Surrogate	%		170 (1)		05/13/94
trachloro-meta-xylene Surrogate	%		79		05/13/94
UTbutylchlorendate (Surrogate)	%		84		05/13/94
Pesticides Prep			05/11/94		



Mg. Mark Liggatt Rige 16

May 16, 1994

PACE Project Number: 540510512

Client Reference: Well Sampling

PMCE Sample Number: Date Collected:

te Received: ent Sample ID:

Parameter

60 0068062 05/10/94 05/10/94 MW 13A

MDL Units

METHOD DATE ANALYZED

0	ANIC	ANALYSIS

OMANIC ANALYSIS				
MOLATILE DOCANICS			8	240
VOLATILE ORGANICS	ug/L	40000	ND	05/14/94
Mathyl chloride (Chloromethane)	ug/L	40000	ND	05/14/94
Mehyl bromide (Bromomethane)	ug/L	40000	NĎ	05/14/94
Vinyl Chloride	ug/L	40000	ND	05/14/94
Chloroethane	ug/L ug/L	20000	ND	05/14/94
Mathylene Chloride	ug/L	40000	ND	05/14/94
Attone	ug/ L	40000	140	•
Carbon Disulfide	ug/L	20000	ND	05/14/94
1. Dichloroethylene	ug/L	20000	ND	05/14/94
1.7-Dichloroethane	ug/L	20000	ND _	05/14/94
1,2-Dichloroethylene (Total)	ug/L	20000	ND 1	05/14/94
Coroform	ug/L	20000	ND	05/14/94
1 Dichloroethane	ug/L	20000	ND	05/14/94
1 Dianio de chane	- 3 / -			
2-Rutanone (MEK)	ug/L	20000	ND	05/14/94
1. 1-Trichloroethane	ug/L	20000	ND	05/14/94
Carpon Tetrachloride	ug/L	20000	ND	05/14/94
/inyl Acetate	ug/L	20000	ND	05/14/94
) Thi orobromomethane	ug/L	20000	ND	05/14/94
1, Dichloropropane	ug/L	20000	ND	05/14/94
	~			
is-1,3-Dichloropropene	ug/L	20000	ND	05/14/94
frechloroethylene	ug/L	20000	670000	05/14/94
:h-orodibromomethane	ug/L	20000	ND	05/14/94
.,1,2-Trichloroethane	ug/L	20000	ND	05/14/94
le rene	üg∕L	20000	ND	05/14/94
ras-1,3-Dichloropropene	ug/L	20000	ND	05/14/94
	•			-6 /1 / /6/
romoform	ug/L	20000	ND	05/14/94
ethy1-2-Pentanone (MIBK)	ug/L	2 000 0	ND	05/14/94
:-Nexanone	ug/L	20000	ND	05/14/94
etrachloroethylene	ug/L	20000	ND	05/14/94
, 2,2-Tetrachloroethane	ug/L	20000	ND	05/14/94
omienė	ug/L	20000	ND	05/14/94
	. 41	20002	MD	05/14/94
hlorobenzene	ug/L	20000	ND	V3/ 14/ 34



Mark Liggatt e 17

May 16, 1994 PACE Project Number: 540510512

Clent	Reference:	Well	Sampling
-------	------------	------	----------

PACE Sample Number:	60 0068062
Date Collected:	05/10/94
De Received:	05/10/94
Clent Sample ID:	MW 13A

Parameter	<u>Units</u>	MDL		METHOD D	ATE ANALYZED
DE ANIC ANALYSIS					
VOLATILE ORGANICS				8240	
E ylbenzene	ug/L	20000	ND		05/14/94
Strene	ug/L	20000	ND		05/14/94
Xylenes (Total)	ug/L	20000	ND		05/14/94
0 ichlorodifluoromethane	ug/L	40000	ND		05/14/94
The chlorofluoromethane	ug/L	20000	ND		05/14/94
Acrolain	ug/L	4000 00	ND		05/14/94
A # ylonitrile	ug/L	400000	ND		05/14/94
I omethane	ug/L	2000 0	ND		05/14/94
Dibromomethane	ug/L	20000	ND		05/14/94
2 hloroethylvinyl ether (mixed)	ug/L	20000	ND		05/14/94
E ylmethacrylate	ug/L	20000	ND		05/14/94
1,2,3-Trichloropropane	ug/L	20000	ND		05/14/94
1 E -Dichloro-2-butene	ug/L	20000	ND		05/14/94
1 - Dichlorobenzene	ug/L	20000	ND		05/14/94
1,4-Dichlorobenzene	ug/L	20000	ND		05/14/94
1_2-Dichlorobenzene	ug/L	20000	ND		05/14/94
1 Dichloroethane-d4 (Surrogate)	%		103		05/14/94
Toluene-d8 (Surrogate)	%		101		05/14/94
(Surrogate)	%		98		05/14/94
PESTICIDES/PCBS				8080/608	
Amelor 1016	ug/L	5.0	ИĎ		05/13/94
Auctor 1221	ug/L	10	ND		05/13/94
4 % clor 1232	ug/L	5.0	ND		05/13/94
Aroclar 1242	ug/L	5.0	ND		05/13/94
Amclor 1248	ug/L	5.0	ND		05/13/94
Amclor 1254	ug/L	5.0	ND		05/13/94
Aroclor 1260	ug/L	5.0	23		05/13/94
Amachlorobiphenyl Surrogate	%		165		05/13/94
[rachloro-meta-xylene Surrogate	%		87		05/13/94
Dibutylchlorendate (Surrogate) Dibutylchlorendate (Surrogate)	%		78 05/11/94		05/13/94

4125 L18



REPORT OF LABORATORY ANALYSIS

h. Mark Liggatt age 18

May 16, 1994

PACE Project Number: 540510512

☐ ient Reference: Well Sampling

CE Sample Number: Date Collected:

60 0068070 05/10/94

05/10/94

te Received: ient Sample ID:

		FW 135			
<u>Units</u>	MDL		METHOD	DATE	ANALYZED

Parameter	<u>Units</u>	MDL	FW 138	METHOD DATE ANALY	ZED
EGANIC ANALYSIS					
VOLATILE ORGANICS				8240	
thyl chloride (Chloromethane)	ug/L	50 0	ND	05/14/	/ G /I
thyl bromide (Bromomethane)	ug/L	500 500	ND	05/14/	
Vinyl Chloride	ug/L	500	ND	05/14/	
Chloroethane	ug/L	500	ND	05/14/	
thylene Chloride	ug/L	250	ND	05/14/	
E etone	ug/L	500	ND	05/14/	
Carbon Disulfide	ug/L	250	NĐ	05/14/	94
1-Dichloroethylene	ug/L	250	ND	05/14/	94
T,1-Dichloroethane	ug/L	250	ND	05/14/	
1.2-Dichloroethylene (Total)	ug/L	250	ND	05/14/	
loroform	ug/L	250	ND	05/14/	
2-Dichloroethane	ug/L	250	ND	05/14/	94
2 Butanone (MEK)	ug/L	250	ND	05/14/	
1,1-Trichloroethane	ug/L	250	ND	05/14/	
Carbon Tetrachloride	ug/L	250	ND	05/14/	
Vinyl Acetate	ug/L	250	ND	05/14/9	
inchlorobromomethane	ug/L	250	ND	05/14/9	
2-Dichloropropane	ug/L	250	ND	05/14/9	9 4
Cds-1,3-Dichloropropene	ug/L	250	ND	05/14/9	94
ichloroethylene	ug/L	250	11000 (2)	05/14/9	94
(m) orodibromomethane	ug/L	250	ND	05/14/9	
1,1,2-Trichloroethane	ug/L	250	ND	05/14/9	
nzene	ug/L	250	ND	05/14/9	
ans-1,3-Dichloropropene	ug/L	250	ND	05/14/9	3 4
Bromoform	ug/Ł	250	ND	05/14/9	9 4
<pre>Methyl-2-Pentanone (MIBK)</pre>	ug/L	250	NO	05/14/9	34
Hexanone	ug/L	250	ND	05/14/9	
Tetrachloroethylene	ug/L	250	ND	05/14/9	
1 2,2-Tetrachloroethane	ug/L	250	ND	05/14/9	
1 uene	ug/L	250	ND	05/14/9) 4
Chlorobenzene	ug/L	250	ND	05/14/9	}4





Mr.	Mark	Liggat	:
ag	e 19		

May 16, 1994 PACE Project Number: 54051051;

Client Reference: Well Sampling

Cilent valetence, act, publica					
ACE Sample Number:			60 0068070)	
Vate Collected:			05/10/94		
Date Received:			05/10/94		
eient Sample ID:			MW 13B		
arameter	Units	MDL	4	METHOD	DATE ANALYZED
MAI SIRE DET					
GREANIC ANALYSIS					
				2242	
VOLATILE ORGANICS				8240	05 /14 /04
Ethylbenzene	ug/L	250	ND		05/14/94
yrene	ug/L	250	ND		05/14/94
Flenes (Total)	ug/L	250	ND		05/14/94
Dichlorodifluoromethane	ug/L	500	ND		05/14/94
Trichlorofluoromethane	ug/L	250	ND		05/14/94
rolein	ug/L	5000	ND	•	05/14/94
Vicin	- 3/				
Acrylonitrile	ug/L	5000	ND		05/14/94
M domethane	ug/L	250	ND		05/14/94
1 bromomethane	ug/L	250	ND		05/14/94
A Chlamathyluthyl athon (mivod)	ug/L	250	ND		05/14/94
2-Chloroethylvinyl ether (mixed)	ug/L	250	ND		05/14/94
5±hylmethacrylate		250	ND		05/14/94
12,3-Trichloropropane	ug/L	230	NU		04/11/01
1,4-Dichloro-2-butene	ug/L	250	ND		05/14/94
1 B-Dichlorobenzene	ug/L	250	ND		05/14/94
15-Dichlorobenzene	ug/L	250	ND		05/14/94
1,2-Dichlorobenzene	ug/L	250	ND		05/14/94
1_2-Dichloroethane-d4 (Surrogate)	%		102		05/14/94
Thuene-d8 (Surrogate)	%		101		05/14/94
the delie-do (Sur i Ogave)					·
1-Bromofluorobenzene (Surrogate)	%		98		05/14/94
TICIDES/PCBS				8080/60	
troclor 1016	ug/L	1.0	ND		05/13/94
Iroclor 1221	ug/L	2.0	ND		05/13/94
1 clor 1232	ug/L	1.0	ND		05/13/94
lector 1242	ug/L	1.0	ND		05/13/94
iroclor 1248	ug/L	1.0	ND		05/13/94
inclor 1254	ug/L	1.0	ND		05/13/94
	 -				A# 154 /A4
roctor 1260	ug/L	1.0	1.81		05/13/94
ecachlorobiphenyl Surrogate	%		190 (1)		05/13/94
rachloro-meta-xylene Surrogate	%		84		05/13/94
Autylchlorendate (Surrogate)	%		84		05/13/94
esticides Prep	••		05/11/94		
entrotata trah					



r. Mark Liggatt Page 20 May 16, 1994

PACE Project Number: 54051051

lient Reference: Well Sampling

PACE Sample Number: Pate Collected: Late Received: Client Sample ID: 60 0068089 05/10/94 05/10/94

ate Collected: ate Received: Client Sample ID: Parameter	Units	MDL	05/10/94 MW 14A	METHOD	DATE ANALYZEI
RGANIC ANALYSIS					
VOLATILE ORGANICS Lethyl chloride (Chloromethane) Lethyl bromide (Bromomethane) Vinyl Chloride Lhloroethane Lethylene Chloride Acetone	ug/L ug/L ug/L ug/L ug/L	500 500 500 500 250 500	ND ND ND ND ND ND	8240	05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
arbon Disulfide .1-Dichloroethylene 1,1-Dichloroethane .2-Dichloroethylene (Total) hloroform 1,2-Dichloroethane	ug/L ug/L ug/L ug/L ug/L ug/L	250 250 250 250 250 250	ND ND ND ND ND ND		05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
-Butanone (MEK) ,1,1-Trichloroethane Carbon Tetrachloride Yinyl Acetate ichlorobromomethane 1,2-Dichloropropane	ug/L ug/L ug/L ug/L ug/L ug/L	250 250 250 250 250 250 250	ND ND ND ND ND ND		05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
is-1,3-Dichloropropene richloroethylene Chlorodibromomethane 1,1,2-Trichloroethane enzene rans-1,3-Dichloropropene	ug/L ug/L ug/L ug/L ug/L ug/L	250 250 250 250 250 250	ND 9600 ND ND ND ND		05/13/94 05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
romoform -Methyl-2-Pentanone (MIBK) 2-Hexanone Tetrachloroethylene 1,2,2-Tetrachloroethane bluene	ug/L ug/L ug/L ug/L ug/L ug/L	250 250 250 250 250 250	ND ND ND ND NO		05/13/94 05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
Chlorobenzene	ug/L	250	ND		05/13/94



Mark Liggatt

May 16, 1994

PACE Project Number: 540510512

05/13/94

lient Reference: Well Sampling

PACE	Sample	Number:
Oate	Collec	ted:

50 0068089 05/10/94

te Received:

05/10/94 MW 14A

lient Sample ID: Parameter

Units MDL		METHOD	DATE ANALYZED
-----------	--	--------	---------------

GANIC ANALYSIS

trylonitrile

VOLATILE ORGANICS hylbenzene yrene Xylenes (Total) ichlorodifluoromethane cichlorofluoromethane Acrolein	ug/L ug/L ug/L ug/L ug/L ug/L	250 250 250 500 250 5000	ND ND ND ND ND ND	8240 05/13/94 05/13/94 05/13/94 05/13/94 05/13/94	
--	--	---	----------------------------------	--	--

5000

ND

domethane	ug/L	250	ND	05/13/94
Dibromomethane	ug/L	250 250	ND ND	05/13/94 05/13/94
Chloroethylvinyl ether (mixed) hylmethacrylate	ug/L ug/L	250 250	ND	05/13/94
1,2,3-Trichloropropane	ug/Ĺ	250	ND	05/13/94
				. = = .

ug/L

4-Dichloro-2-butene 3-Dichlorobenzene 1,4-Dichlorobenzene 2-Dichlorobenzene 2-Dichloroethane-d4 (Surrogate)	ug/L ug/L ug/L ug/L %	250 250 250 250	ND ND ND ND 100 100	05/13/94 05/13/94 05/13/94 05/13/94 05/13/94
Toluene-d8 (Surrogate)	76		100	05/13/

· · · · · · · · · · · · · · · ·				
Bromofluorobenzene (Surrogate)	%	98	1999	05/13/94

-promotitioropenzene (autrogate)	70		30		00/10/51
PESTICIDES/PCBS				\$080/60 8	
Aroclor 1016	ug/L	1.0	ND	•	05/13/94
roclor 1221	ug/L	2.0	ND		05/13/94
Aroclor 1232	ug/L	1.0	ND		05/13/94
Aroclar 1242	ug/L	1.0	ND		05/13/94
Proclor 1248	ug/L	1.0	ND		05/13/94
roctor 1254	ug/L	1.0	ND		05/13/94
Aroclor 1260	ug/L	1.0	ND		05/13/94
ecachlorobinhenvi Surrogate	~5 <i>,</i> ~		116		05/13/94

05/13/94 81 Tetrachloro-meta-xylene Surrogate 05/13/94 65 Dibutylchlorendate (Surrogate) 05/11/94

esticides Prep

05/13/94

Y-17-94 TUE 06:46

PACE, INC

FAX NO. 9135991759



REPORT OF LABORATORY ANALYSIS

r. Mark Liggatt age 22

May 16, 1994

PACE Project Number: 54051051:

lient Reference: Well Sampling

ethyl bromide (Bromomethane)

PACE Sample Number:

Date Collected:

ate Réceived: Nient Sample ID:

Vinyl Chloride

60 0068097 05/10/94 05/10/94

MW 15A Parameter Units MDL METHOD DATE ANALYZED RGANIC ANALYSIS VOLATILE ORGANICS 8240 **ethyl chloride (C**hloromethane) 2000 NO ug/L 05/13/94

2000

2500

ND

NO

05/13/94 **C**hloroethane ug/L 2500 NN 05/13/94 ethylene Chloride ug/L 1200 ND 05/13/94 cetone ug/L 2500 ND 05/13/94 arbon Disulfide 1200 ND 05/13/94 uq/L

ug/L

ug/L

L1-Dichloroethylene 1200 ND ug/L 05/13/94 1,1-Dichloroethane 1200 ND uq/L 05/13/94 **L2-Dichloroethylene** (Total) 1200 24000 ug/L 05/13/94 hloroform 1200 ug/L ND 05/13/94 .Z-Dichloroethane 1200 ND ug/L 05/13/94

Butanone (MEK) 1200 ND 05/13/94 ug/L 1,1-Trichloroethane 1200 ug/L ND 05/13/94 Carbon Tetrachloride ug/L 1200 ND 05/13/94 Yinyl Acetate 1200 ND ug/L 05/13/94 ichlorobromomethane ug/L 1200 ND 05/13/94

2-Dichloropropane 1200 ND ug/L 05/13/94 #s-1,3-Dichloropropene 1200 05/13/94 ug/L ND tichloroethylene ug/L 1200 37000 05/13/94 **Chlorodibromomethane** 1200 ND ug/L 05/13/94 L1,2-Trichloroethane 1200 ug/L ND 05/13/94

ug/L 1200 ND 05/13/94 Fans-1,3-Dichloropropene ug/L 1200 ND 05/13/94 romoform 1200 ND 05/13/94 ug/L Methyl-2-Pentanone (MIBK) 1200 ND ug/L 05/13/94 Z-Hexanone ug/L 1200 ND 05/13/94 Tetrachloroethylene ND 1200 ug/L 05/13/94

1,2,2-Tetrachloroethane ug/L 1200 ND 05/13/94 Nuene ND 1200 05/13/94 ug/L lorobenzene ug/L 1200 ND 05/13/94



. Mark Liggatt ge 23

May 16, 1994 PACE Project Number: 540510512

Client Reference: Well Sampling

Date Collected: Date Received: Date Received: Date Received:

60 0068097 05/10/94 05/10/94

<u>rame</u>ter

MW 15A <u>Units</u> MOL METHOD DATE ANALYZED

				11511100	DATE ANALIZED
GAGANIC ANALYSIS					
VOLATILE ORGANICS				8240	
Ethylbenzene	ug/L	1200	ND	0240	05/19/04
Syrene	ug/L	1200	ND		05/13/94
河enes (Total)	ug/L	1200	ND		05/13/94
Dichlorodifluoromethane	ug/L	2500	ND		05/13/94 05/13/94
Michlorofluoromethane	ug/L	1200	ND		05/13/94
Agrolein	ug/L	25000	ND		05/13/94
A <u>crylonitrile</u>	ug/L	25000	ND		05/13/94
1 iomethane	ug/L	1200	ND		05/13/94
C Toromomethane	ug/L	1200	ND		05/13/94
2-Chloroethylvinyl ether (mixed)	ug/L	1200	ND		05/13/94
Emylmethacrylate	ug/L	1200	ND		05/13/94
12,3-Trichloropropane	ug/L	1200	ND		05/13/94
1_4-Dichloro-2-butene	ug/L	1200	ND		05/13/94
1 3 - Oich i orobenzene	ug/L	1200	ND		05/13/94
1 - Dichlorobenzene	ug/L	1200	ND		05/13/94
1,2-Dichlorobenzene	ug/L	1200	ND		05/13/94
1 Dichloroethane-d4 (Surrogate)	%		93		05/13/94
Teuene-d8 (Surrogate)	%		106		05/13/94
4-Bromofluorobenzene (Surrogate)	%		92		05/13/94
PATICIDES/PCBS				8080/608	
Aroclor 1016	ug/L	1.0	ND	• • •	05/13/94
Amelor 1221	ug/L	2.0	ND		05/13/94
hactor 1232 troclor 1242	ug/L	1.0	ND		05/13/94
troclor 1242	ug/L	1.0	ND		05/13/94
unclor 1254	ug/L	1.0	ND		05/13/94
	ug/L	1.0	ND		05/13/94
iroclor 1260	ug/L	1.0	1.08		05/13/94
legachlorobiphenyl Surrogate	%		168 (1)		05/13/94
arachloro-meta-xylene Surrogate	%		77		05/13/94
Houtylchlorendate (Surrogate)	%		87		05/13/94
esticides Prep			05/11/94		,,

FAX NO. 9135991759

#153 PØ6 P. 24



REPORT OF LABORATORY ANALYSIS

Mark Liggatt

FOOTNOTES for pages 1 through 25

May 16, 1994 PACE Project Number: 54051051

☐ ient Reference: Well Sampling

MDL

Method Detection Limit

Not detected at or above the MDL.

The Surrogate recovery value exceeded the established

laboratory control limit value.

Compound concentration exceeds the calibration range of the instrument.

QUALITY

4127 LAL

FAX NO. 9135881759

P. 25



REPORT OF LABORATORY ANALYSIS

r. Mark Liggatt age 27

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

Client Reference: Well Sampling

ESTICIDES/PCBS

Batch: 60 31803

amples: 60 0067996, 60 0068003, 60 0068011, 60 0068020, 60 0068046 60 0068054, 60 0068062, 60 0068070, 60 0068089, 60 0068097

ETHOD BLANK:

Parameter Proclor 1016 Proclor 1221 Proclor 1232 Proclor 1242 Proclor 1248 Proclor 1254	Units ug/L ug/L ug/L ug/L ug/L ug/L ug/L	MDL 1.0 2.0 1.0 1.0 1.0	Method Blank ND ND ND ND ND ND
roclor 1260	ug/L	1.0	ND
ecachlorobiphenyl Surrogate	%		185 (1)
Tetrachloro-meta-xylene Surrogate	%		107
Dibutylchlorendate (Surrogate)	%		78

LABORATORY CONTROL SAMPLE AND CONTROL SAMPLE DUPLICATE:

			Reference	Dup1
arameter	Uni <u>ts</u>	MDL.	<u> Value Rec</u>	
Aroclor 1242	ug/L	1.0	5.0 92	
Aroclor 1260	ug/L	1.0	5.0 99	% 96%

9608 Loiret Boulevard Lenexa, KS 66216 TEL: 513-699-5865 FAX: 913-599-1759

#153 P08

FAX NO. 9135991759

P. 28



REPORT OF LABORATORY ANALYSIS

ir. Mark Liggatt age 28

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105.

Client Reference: Well Sampling

ESTICIDES/PCBS Batch: 60 31810 amples: 60 0068038

METHOD BLANK:

lindto	MOI	Method
		Blank ND
ug/L		ND
ug/L	1.0	ND
%		240 (1)
		82
%		98
	ug/L ug/L ug/L ug/L	ug/L 1.0 ug/L 2.0 ug/L 1.0 ug/L 1.0 ug/L 1.0 ug/L 1.0 ug/L 1.0

BORATORY CONTROL SAMPLE AND CONTROL SAMPLE DUPLICATE:

Parameter Coclor 1242 Coclor 1260	<u>Units</u> ug/L ug/L	MDL 1.0 1.0	Reference <u>Value</u> <u>Recv</u> 5.0 92% 5.0 99%	104%	RPD 12. 3.
					_

9608 Lairet Baulevard Lonoxo, KS 66218 TEL: 913-596-5665 FAX: 913-589-1759

PACE, INC

FAX NO. 9135991759

P. 27



REPORT OF LABORATORY ANALYSIS

Mr. Mark Liggatt Page 29 QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

Client Reference: Well Sampling

VOLATILE ORGANICS Batch: 60 31784 Samples: 60 0068020

_METHOD BLANK:	1		
Size How Welling	1		Method
Parameter	Units	MDL	Blank_
Methyl chloride (Chloromethane)	ug/L	10	ND
Methyl bromide (Bromomethane)	ug/L	10	ND
Vinyl Chloride	ug/L	10	NO
Chloroethane	ug/L	10	ND
Methylene Chloride	ug/L	5	5
Acetone	ug/L	10	ND
ACETOTIC	~ 3 / ~		
Carbon Disulfide	ug/L	5	מא
1.1-Dichloroethylene	ug/L	5	ND
1.1-Dichloroethane	ug/L	5	ND
1,2-Dichloroethylene (Total)	ug/L	5 5 5 5 5 5	ФИ
_Chloroform	ug/L	5	ND
1,2-Dichloroethane	ug/L	5	ND
			
2-Butanone (MEK)	ug/L	5	ND
m1,1,1-Trichloroethane	ug/L	5	ND
Carbon Tetrachloride	ug/L	5	ND
Vinyl Acetate	ug/L	55555	ND
_Dichlorobromomethane	ug/L	5	ND
1,2-Dichloropropane	ug/L	5	ND
1,2-0 ichioi opi opane	~\$/ -	•	
Cis-1,3-Dichloropropene	ug/L	5	ND
mTrichloroethylene	ug/L	5	ND
Chlorodibromomethane	ug/L	5 5 5 5 5	ND
1,1,2-Trichloroethane	ug/L	5	ND
Benzene	ug/L	5	ND
Trans-1,3-Dichloropropene	ug/L	5	ND
	•		
Bromoform	ug/L	5	ND
#4-Methy1-2-Pentanone (MIBK)	ug/L	5	23
2-Hexanone	ug/L	5	ND
Tetrachloroethylene	ug/L	5 5 5 5 5	МD
1,1,2,2-Tetrachloroethane	ug/L	5	ND
Toluene	ug/L	5	ND
Chlorobenzene	ug/L	5	ND
Ethylbenzene	ug/L	5	ND

PACE, INC

FAX NO. 9135991758

P. 28



REPORT OF LABORATORY ANALYSIS

Mr. Mark Liggatt Page 30

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

fifent Reference: Well Sampling

VOLATILE ORGANICS Batch: 60 31784 Bamples: 60 0068020

METHOD BLANK:

Parameter Styrene Lylenes (Total) Sichlorodifluoromethane Trichlorofluoromethane Acrolein Crylonitrile	Units ug/L ug/L ug/L ug/L ug/L	MDL 5 5 10 5 100 100	Method Blank ND ND ND ND ND ND
Iodomethane ibromomethane -Chloroethylvinyl ether (mixed) Ethylmethacrylate 1,2,3-Trichloropropane 4-Dichloro-2-butene	ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5	ND ND ND ND ND ND
1,3-Dichlorobenzene 1,4-Dichlorobenzene 2-Dichlorobenzene 1,2-Dichloroethane-d4 (Surrogate) Toluene-d8 (Surrogate) Bromofluorobenzene (Surrogate)	ug/L ug/L ug/L % %	5 5 5	ND ND ND 94 97

SPIKE AND SPIKE DUPLICATE:

Parameter 1-Dichloroethylene	<u>Units</u> ug/L	<u>MDL</u> 5	600068020 PZ 4A ND	<u>Spike</u> 50.00	Spike Recv 53%	Spike Dupl Recy 48%	<u>RP</u> 10'
Trichloroethylene Benzene Tuene Galorobenzene	ug/L ug/L ug/L ug/L	5 5 5 5	88 ND ND ND	50.00 50.00 50.00 50.00	(2) 112% 96% 10 8% 113%	(3) 108% 93% 104% 109%	4: 3: 4: 4:



r. Mark Liggatt Page 31

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 54051051

lient Reference: Well Sampling

VOLATILE ORGANICS Batch: 60 31784 mples: 60 0068020

Parameter Methyl chloride (Chloromethane) Methyl bromide (Bromomethane) Vinyl Chloride Chloroethane Methylene Chloride Acetone	Units ug/L ug/L ug/L ug/L ug/L ug/L	MDL 10 10 10 10	50 1 50 1 50 1 50 1 50 1 50 1	ecv 52% 10% 25% 10% 09% 81%
Carbon Disulfide 1,1-Dichloroethylene 1,1-Dichloroethane 1,2-Dichloroethylene (Total) Chloroform 1,2-Dichloroethane	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5	50 1 50 1 100 1 50 1	173% 121% 100% 112% 102% 108%
2-Butanone (MEK) 1,1,1-Trichloroethane Carbon Tetrachloride Dichlorobromomethane 1,2-Dichloropropane Cis-1,3-Dichloropropene	ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5	50 50 50 50	104% 101% 103% 102% 101%
Trichloroethylene Chlorodibromomethane I,1,2-Trichloroethane Benzene Trans-1,3-Dichloropropene Bromoform	ug/L ug/L ug/L ug/L ug/L	5555555	50 50 50 50 50	100% 98% 99% 105% 100% 101%
4-Methyl-2-Pentanone (MIBK) Tetrachloroethylene 1,1,2,2-Tetrachloroethane Toluene Chlorobenzene Ethylbenzene	ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5	50 50 50 50 50 50	36% 98% 96% 100% 98% 98%
Styrene Xylenes (Total)	ug/L ug/L	5 5	50 150	102% 97%





Mark Liggatt Page 32

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 54051051;

ient Reference: Well Sampling

VOLATILE ORGANICS

Batch: 60 31792 mples: 60 0067996, 60 0068003, 60 0068011, 60 0068054, 60 0068089

60 0068097

THOD BLANK:			Method
Parameter	Units	<u>MDL</u> 10	<u>Blank</u> ND
sthyl chloride (Chloromethane)	ug/L ug/L	10	ND
Methyl bromide (Bromomethane) Vinyl Chloride	ug/L	10	ND
injorpethane	ug/L	10	ND
ethylene Chloride	ug/L	5 10	ND ND
Acetone	ug/L	10	
arbon Disulfide	ug/L	5 5 5 5 5	ND ND
1.1-Dichloroethylene	ug/L	5 E	ND ND
1,1-Dichloroethane	ug/L ug/L	5 5	ND
2-Dichloroethylene (Total)	ug/L	5	ND
hloroform 1,2-Dichloroethane	ug/L	5	ND
1,2-DICHIOFOSCHARE		•	MO
-Butanone (MEK)	ug/L	5 5	ND ND
,1,1-Trichloroethane	ug/L ug/L	5 5	ND
Carbon Tetrachloride	ug/L ug/L	555555	ND
Minyl Acetate	ug/L	5	ND
ichlorobromomethane 1.2-Dichloropropane	ug/L	5	ND
		•	МĎ
is-1,3-Dichloropropene	ug/L	5 5	ND
Frichloroethylene	ug/L ug/L	5	ND
Chlorodibromomethane	ug/L	5	ND
1,1,2-Trichlorosthane Benzane	ug/L	5 5 5 5	ND
frans-1,3-Dichloropropene	ug/L	,5	ND
	ug/L	5	ND
Bromoform 4-Mathyl-2-Pentanone (MIBK)	ug/L	5	17
2-Hexanone	ug/L	5555555	ND
-Tetrachloroethylene	ug/L	5	ND
1,1,2,2-Tetrachloroethane	ug/L	5	ND ND
Toluene	ug/L	5	ואט
Chlorobenzene	ug/L	5	ND



. Mark Liggatt Page 33

QUALITY CONTROL DATA

May 16, 1994 PACE Project Number: 54051051

lent Reference: Well Sampling

VOLATILE ORGANICS

amples: 60 0067996, 60 0068003, 60 0068011, 60 0068054, 60 0068089

60 0068097

Parameter Ithylbenzene Styrene Xylenes (Total) Dichlorodifluoromethane Trichlorofluoromethane Acrolein	<u>U</u> nits ug/L ug/L ug/L ug/L ug/L	MDL 5 5 5 10 5 100	Method Blank ND ND ND ND ND ND
Acrylonitrile Iodomethane Dibromomethane 2-Chloroethylvinyl ether (mixed) Ethylmethacrylate 1,2,3-Trichloropropane	ug/L ug/L ug/L ug/L ug/L	100 5 5 5 5 5	ND ND ND ND ND ND
1.4-Dichloro-2-butene 1.3-Dichlorobenzene 1.4-Dichlorobenzene 1.2-Dichlorobenzene 1.2-Dichloroethane-d4 (Surrogate) Toluene-d8 (Surrogate)	ug/L ug/L ug/L ug/L %	5 5 5 5	ND ND ND ND 101
4-Bromofluorobenzene (Surrogate)	%		100

SPIKE AND SPIKE DUPLICATE:		•	600068020		Spi ke	Sp1ke Dupl
Parameter 1,1-Dichloroethylene	Units ug/L	MOL 5	PZ 4AND	<u>Spike</u> 50.00	8ecv 53% (4)	Recv 48% (5)
Trichloroethylene Benzene Toluene Chlorobenzene	ug/L ug/L ug/L ug/L	5 5 5	88 DD DN ON	50.00 50.00 50.00 50.00	112% 96% 108% 113%	108% 93% 104% 109%

#155 P05

17-94 TUE 06:55

PACE, INC

FAX NO. 9135991759

P. 37



REPORT OF LABORATORY ANALYSIS

Mark Liggatt

(1)

(3)

FOOTNOTES for pages 27 through 38 May 16, 1994 PACE Project Number: 540510511

Wient Reference: Well Sampling

Method Detection Limit MOL

Not detected at or above the MDL.

Relative Percent Difference

The Surrogate recovery value exceeded the established

laboratory control limit value.

The percent recovery for 1,1-Dichloroethene is below the laboratory control established control limit.

The percent recovery for 1,1-Dichloroethene is below the laboratory control

established control limit. The percent recovery for 1,1-Dichlorosthene is below the laboratory control

established control limit. The percent recovery for 1,1-Dichloroethene is below the laboratory control

established control limit. The percent recovery for 1,1-Dichloroethene is below the laboratory control

established control limit.

The percent recovery for 1,1-Dichloroethene is below the laboratory control

established control limit.



r. Mark Liggatt Tage 38 QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

filent Reference: Well Sampling

VOLATILE ORGANICS _ Batch: 60 31795

amples: 60 0068038, 60 0068046, 60 0068062, 60 0068070, 60 0068100

LABORATORY CONTROL SAMPLE:

Tarameter Methyl chloride (Chloromethane) White the state of the state	Units ug/L ug/L ug/L ug/L ug/L ug/L	MDL 10 10 10 10 10 5	50 50 50 50	Recy 152% 110% 125% 110% 109% 81%
Carbon Disulfide .1-Dichloroethylene .1-Dichloroethane 1,2-Dichloroethylene (Total) Chloroform .2-Dichloroethane	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5 5	50 1 50 3 100 1 50 1	73% 121% 100% 112% 102% 108%
2-Butanone (MEK) 1,1-Trichloroethane arbon Tetrachloride Dichlorobromomethane 1,2-Dichloropropane is-1,3-Dichloropropene	ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5	50 1 50 1 50 1	28% 04% 01% 03% 02% 01%
Trichloroethylene hlorodibromomethane ,1,2-Trichloroethane Benzene Trans-1,3-Dichloropropene romoform	ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5 5	50 50 50 1 50 1	00% 98% 99% 05% 00% 01%
4-Methyl-2-Pentanone (MIBK) etrachloroethylene ,1,2,2-Tetrachloroethane Toluene Chlorobenzene thylbenzene	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5 5	50 50 50 1 50	36% 98% 96% 00% 98% 98%
Styrene Wylenes (Total)	ug/L ug/L	5 5		02% 97%



Mr. Mark Liggatt Page 37

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

Client Reference: Well Sampling

VOLATILE ORGANICS Batch: 60 31795

Samples: 60 0068038, 60 0068046, 60 0068062, 60 0068070, 60 0068100

METHOD BLANK:

Units ug/L ug/L	MDL 5 5	Blank
	5	110
		ND
43/	5	ND
ug/L	10	ND
	5	ND
	100	ND
ug/L	100	ND
ua/I	5	ND
		ND
	5	ND
	5	ND
	5	ND
ug/L	5	ND
ua/L	5	ND
	5	ND
ug/L		ND
%		103
		101
%		100
	ug/L ug/L ug/L ug/L ug/L ug/L ug/L ug/L	ug/L 10 ug/L 100 ug/L 100 ug/L 5

SPIKE AND SPIKE DUPLICATE:

	i					201KE	
	[600068020		Spike	Dupl	
Parameter	Units	<u>MDL</u> 5	PZ 4A	Spike	Řecv	Recy	F
1,1-Dichloroethylene	ug/L	5	ND	50,00	53%	48%	Ī
	•				(6)	(7)	
Trichloroathylene	ug/L	5	88	50.00	Ì12%	108%	
Benzene	ug/L	5	ND	50.00	96%	93%	
foluene	ug/L	5	ND	50.00	108%	104%	
Thlorobenzene	ug/L	5	ND	50.00	113%	109%	

9608 Loiret Boulevard LINIERA, KS 55219 TEL: 813-599-5665 FAX: \$13-599-1759

Caile

PACE. INC

FAX NO. 9135991759





REPORT OF LABORATORY ANALYSIS

r. Mark Liggatt Page 34

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

llient Reference: Well Sampling

VOLATILE ORGANICS

Batch: 60 31792 Samples: 60 0067995, 60 0068003, 60 0068011, 60 0068054, 60 0068089

60 0068097

LABORATORY CONTROL SAMPLE:			Reference	
Parameter Methyl chloride (Chloromethane) Methyl bromide (Bromomethane) Vinyl Chloride Chloroethane Methylene Chloride Acetone	<u>Units</u> ug/L ug/L ug/L ug/L ug/L	MDL 10 10 10 10 5	Value 50 50 50 50 50 50	Recy 152% 110% 125% 110% 109% 81%
Carbon Disulfide 1,1-Dichloroethylene 1,1-Dichloroethane 1,2-Dichloroethylene (Total) Chloroform 1,2-Dichloroethane	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5	50 50 50 100 50 50	173% 121% 100% 112% 102% 108%
2-Butanone (MEK) 1,1,1-Trichloroethane Carbon Tetrachloride Dichlorobromomethane 1,2-Dichloropropane Cis-1,3-Dichloropropene	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5	50 50 50 50 50 50	28% 104% 101% 103% 102% 101%
Trichloroethylene Chlorodibromomethane 1,1,2-Trichloroethane Benzene Trans-1,3-Dichloropropene Bromoform	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5 5 5	50 50 50 50 50 50	100% 98% 99% 105% 100% 101%
4-Methyl-2-Pentanone (MIBK) Tetrachloroethylene I,1,2,2-Tetrachloroethane Toluene Chlorobenzene Ethylbenzene	ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5	50 50 50 50 50	36% 98% 96% 100% 98% 98%
Styrene	ug/L	5	50	102%

#155 PØ9

-17-94 TUE 08:53

PACE, INC

FAX NO. 9135991759

P. 33



REPORT OF LABORATORY ANALYSIS

r. Mark Liggatt Page 35

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

lient Reference: Well Sampling

VOLATILE ORGANICS

Batch: 60 31792

Samples: 60 0067996, 60 0068003, 60 0068011, 60 0068054, 60 0068089

60 0068097

ABORATORY CONTROL SAMPLE:

Parameter Xylenes (Total) Units ug/L

MOL

Reference Value 150

Recy 97%



r. Mark Liggatt age 36

QUALITY CONTROL DATA

May 16, 1994

PACE Project Number: 5405105

flient Reference: Well Sampling

VOLATILE ORGANICS

Batch: 60 31795 Samples: 60 0068038, 60 0068046, 60 0068062, 60 0068070, 60 0068100

DOHTSN	BLANK:
--------	--------

Parameter Methyl chloride (Chloromethane) Methyl bromide (Bromomethane) Minyl Chloride Chloroethane Methylene Chloride Acetone	Units ug/L ug/L ug/L ug/L ug/L ug/L	MDL 10 10 10 10 5	Method Blank ND ND ND ND ND ND
Carbon Disulfide 1,1-Dichlorosthylene 1,1-Dichlorosthane 1,2-Dichlorosthylene (Total) Chloroform 1,2-Dichlorosthane	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5	ND ND ND ND ND ND
2-Butanone (MEK) 1,1,1-Trickloroethane Carbon Tetrachloride Vinyl Acetate Dichlorobromomethane 1,2-Dichloropropane	ug/L ug/L ug/L ug/L ug/L ug/L	5 5 5 5 5 5 5	ND ND ND ND ND ND
Cis-1,3-Dichloropropene Trichloroethylene Chlorodibromomethane 1,1,2-Trichloroethane Benzene Trans-1,3-Dichloropropene	ug/L ug/L ug/L ug/L ug/L ug/L	555555	ND ND ND ND ND ND
Bromoform 4-Methyl-2-Pentanone (MIBK) 2-Hexanone Tetrachloroethylene 1,1,2,2-Tetrachloroethane Toluene	ug/L ug/L ug/L ug/L ug/L ug/L	55555 5 5	ND ND ND ND ND ND
Chlorobenzene Ethylbenzene	ug/L ug/L	5 5	ND ND



Mark Liggatt ge 24

May 16, 1994 PACE Project Number: 540510512

Client Reference: Well Sampling

PECE Sample Number:	60 0068100
Date Collected:	05/10/94
Data Received:	05/10/94
Client Comple ID:	Tuin

Pace Sample Number: Date Collected: Date Received: Collent Sample ID: Parameter	<u>Units</u>	MDL	60 0068100 05/10/94 05/10/94 Trip Blank	METHOD	DATE ANALYZED
OPGANIC ANALYSIS	•• *				
VOLATILE ORGANICS				8240	
Methyl chloride (Chloromethane)	ug/L	10	ND	_	05/14/94
Mathyl bromide (Bromomethane)	ug/L	10	NO		05/14/94
Vanyl Chloride	ug/L	10	ND		05/14/94
Chioroethane	ug/L	10	ND		05/14/94
<u>Me</u> thylene Chloride	ug/L	5	ND		05/14/94
Aptone	ug/L	10	ND		05/14/94
Carbon Disulfide	ug/L	5	ND		05/14/94
I_l-Dichloroethylene	ug/L	5 5 5 5 5	ND		05/14/94
1 Dichloroethane	ug/L	5	ND		05/14/94
1 72-Dichloroethylene (Total)	ug/L	5	ND		05/14/94
Chloroform	ug/L	5	ND		05/14/94
12-Dichloroethane	ug/L	5	ND		05/14/94
2-Butanone (MEK)	ug/L	5	ND		05/14/94
1,1,1-Trichloroethane	ug/L	5 5 5 5 5 5	ND		05/14/94
Carbon Tetrachloride	ug/L	5	ND		05/14/94
Veryl Acetate	ug/L	5	ND		05/14/94
Dichlorobromomethane	ug/L	5	ND		05/14/94
1.2-Dichloropropane	ug/L	5	ND		05/14/94
CE-1,3-Dichloropropene	ug/L	5	ND		05/14/94
Trichloroethylene	ug/L	5 5 5 5	ND		05/14/94
Chi orodibromomethane	ug/L	5	ND		05/14/94
1 ,2-Trichloroethane	ug/L	<u>5</u>	ND		05/14/94
BEnzene	ug/L	5	ND		05/14/94
Trans-1,3-Dichloropropene	ug/L	5	ND		05/14/94
Banoform	ug/L	5	ND		05/14/94
4-Methy1-2-Pentanone (MIBK)	ug/L	5	23		05/14/94
2 <u>-H</u> exanone	ug/L	5 5 5 5 5 5 5	ND		05/14/94
[rachlorosthylene	ug/L	5	ND		05/14/94
1 ,2,2-Tetrachloroethane	ug/L	5	ND		05/14/94
[o] uene	ug/L	5	ND		05/14/94
Cleorobenzene	ug/L	5	ND		05/14/94

9608 Lairet Saulevard Lenexa, K\$ 66219 TEL: 913-009-5565 FAX: 813-689-1759

An Equal Opportunity Employer



Mark Liggatt 25

May 16, 1994

PACE Project Number: 540510512

:lient Reference: Well Sampling

Sample Number:

)ate Collected:

e Received:

ent Sample ID: **Parameter**

60 0068100 05/10/94 05/10/94 Trip

MDL

METHOD DATE ANALYZED **Blank**

DR ANIC ANALYSIS

DREANIC ANALYSIS					•
IOLATTI F ODČANICS				8240	
OLATILE ORGANICS	ug/L	5	ND		05/14/94
Ethylbenzene	ug/L	5	ND		05/14/94
Starene	ug/L	5 5 5	ND		05/14/94
(yrenes (Total)	ug/L	10	ND		05/14/94
)ichlorodifluoromethane	ug/L	5	ND		05/14/94
[rachlorof] uoromethane	ug/L	100	ND		05/14/94
Action	ug/L	100	110		447 = 17
	ug/L	100	ND		05/14/94
Acceylonitrile			ND		05/14/94
Ideomethane	ug/L	š	ND		05/14/94
) for omomethane	ug/L	Ĕ	ND		05/14/94
2-Chloroethylvinyl ether (mixed)	ug/L	5 5 5 5	ND		05/14/94
E m ylmethacrylate	ug/L	S r	ND		05/14/94
1, 3 ,3-Trichloropropane	ug/L	5	NU		00/ 1/ 5.
	/1	E	ND		05/14/94
l d-Dichloro-2-butene	ug/L		ND		05/14/94
1 Dichlorobenzene	ug/L	5 5 5 5	ND		05/14/94
1,4-Dichlorobenzene	ug/L	5	ND		05/14/94
1,2-Dichlorobenzene	ug/L	3			05/14/94
l - Dichloroethane-d4 (Surrogate)	% %		106		05/14/94
Touene-d8 (Surrogate)	%		102		VV) 17/ 37
4_Bromofluorobenzene (Surrogate)	%		102		05/14/94

Units

These data have been reviewed and are approved for release.

. Bolinger 1 R. Hudson

ager, Organic Chemistry

APPENDIX D

MATERIAL SAFETY DATA SHEET

UNISON TRANSFORMER SERVICES, INC. 1338 Hundred Oaks Drive Charlotte, NC 28217 1 800 544-0030

EFFECTIVE DATE: October 15, 1987

POD MSDS 811-1

IMPORTANT: UNISON urges each customer or recipient of this MSDS to study it carefully to become aware of and understand the hazards associated with the material. The reader should consider consulting reference works or individuals who are experts in ventilation, toxicology or fire prevention, as necessary or appropriate, to use and understand the data contained in this MSDS.

> To Promote safe handling, it is recommended that each Customer. notify its employees, agents, and contractors of the information on this Material Safety Data Sheet.

I. IDENTIFICATION

PRODUCT NAME:

MO-X-PCB

CHEMICAL NAME:

Light naphthenic distillate, petroleum, with PCB

CHEMICAL FAMILY:

Petroleum Electrical Insulating Oil with PCB

FORMULA:

Mixture

MOLECULAR WEIGHT:

Mixture

SYNONYMS:

Mineral Oil which contains PCBs

COPYRIGHT 1987 - UNISON TRANSFORMER SERVICES, INC. A Subsidiary of Union Carbide Corporation

******* II. PHYSICAL DATA (Determined on typical material) ************** BOILING POINT, 760 mm Hg: CA 250 Deg. C SPECIFIC GRAVITY $(H_2O = 1):$ 0.87 at 25/25 Deg. C FREEZING POINT: Less than -45 Deg. C VAPOR PRESSURE AT 20 Deg. C: Less than 1 mm Hg VAPOR DENSITY (air = 1): Greater than 1 EVAPORATION RATE (Butyl Acetate = 1): Less than 1 SOLUBILITY IN WATER % by wt: Insoluble APPEARANCE AND ODOR: Clear, nearly colorless liquid, aromatic odor III. INGREDIENTS MATERIALS Concentration TLV UNITS Mineral Oil Approx. 98.5-99.5 % See Section V CAS NO: 64742-53-6,64742-45-6, 64741-96-4, or 64742-52-5 2,6-di-tertiary-butylpara-cresol Approx. 0.5 % None Established CAS NO: 128-37-0 Polychlorinated Biphenyls Approx. 50-10000 ppm See Section V CAS NO: 1336-36-3 IV. FIRE AND EXPLOSION HAZARD DATA FLASH POINT (test method): Greater than 293 Deg. F (Cleveland open cup ASTM D 92). FLAMMABLE LIMITS IN AIR, by volume: LOWER 0.9 estimated UPPER 7.0 estimated EXTINGUISHING MEDIA: Use water spray, carbon dioxide, dry chemical, alcohol-type or universal-type foams applied by manufacturer's recommended technique. SPECIAL FIRE FIGHTING PROCEDURES: Wear self-contained breathing apparatus and standard fire-fighter wearing apparel: toxic and irritating vapors can be evolved. Do not spray a solid stream of water or foam directly into a pool of hot burning liquid

as this may cause frothing and may intensify the fire.

(FIRE AND EXPLOSION HAZARD DATA continued on next page)

UNUSUAL FIRE AND EXPLOSION HAZARDS:

In extreme fire conditions, this material may present a floating fire hazard. See Section VI for products of combustion.

V. HEALTH HAZARD DATA

TLV (SOURCE): PCBs

42% chlorine, 8-hr TWA 1.0 mg/m3 - Skin * (ACGIH) 2 mg/m3 short-term exposure limit - Skin * (ACGIH) 54% chlorine, 8 hr TWA 0.5 mg/m3 - Skin* (ACGIH) lmg/m3 short term exposure limit - Skin (ACGIH)

Oil Mist,

Mineral

8 hr TWA 5.0 mg/m3 (ACGIH), STEL 10 mg/m3.

Skin absorption may add to the overall exposure. Avoid skin contact.

EFFECTS OF SINGLE OVEREXPOSURE:

SWALLOWING: Moderate toxicity, irritating to throat, may cause lethargy,

nausea, vomiting, and diarrhea. Aspiration of light hydrocarbon or subsequent vomiting may cause pneumonitis.

SKIN ABSORPTION: Moderately toxic. Prolonged or widespread contact may

result in the absorption of potentially harmful amounts of

material.

INHALATION: Mists or vapors from heated material are irritating to nose,

throat and lungs.

SKIN CONTACT: Brief contact causes minimal irritation.

EYE CONTACT: Liquid or vapor may cause minor irritation, depending on the

concentration of vapor and duration of exposure.

EFFECTS OF REPEATED

OVEREXPOSURE: Liver injury, chloracne, skin lesions, skin sensitization.

Laboratory studies have shown Polychlorinated Biphenyls to be

an animal carcinogen.

MEDICAL CONDITIONS

AGGRAVATED BY OVEREXPOSURE: Persons who are prone to acne or seborrhea are

susceptible to blockage of hair follicles by insoluble oils. Folliculitis may develop.

Comedones and perifollicular papules and pustules (oil boils) may occur. These effects should rapidly clear upon cessation of exposure. Good housekeeping and personal hygiene should prevent

these effects.

(HEALTH HAZARD DATA continued on next page)

SIGNIFICANT LABORATORY DATA WITH POSSIBLE RELEVANCE TO HUMAN HEALTH HAZARD EVALUATION

HUMAN HEALTH HAZARD EVALUATION: PCBs have been identified as hazardous chemicals under criteria of the OSHA Hazard Communication Standard (29 CFR Part 1910.1200). PCBs have been listed in the International Agency for Research on Cancer (IARC) Monograph (1982) 'Group 2B' and in the National Toxicology Program (NTP) Annual Report on Carcinogens (Third).

Several studies report a wide spectrum of immunotoxic effects of PCBs on rats and mice, including thymic, lymph node and splenic atrophy, suppression of antibody responses, decreased resistance to infections, depression of T-cell responsiveness to mitogens and suppression of delayed hypersensitivity.

OTHER EFFECTS

OF OVEREXPOSURE: PCBs are readily absorbed into the body by all routes of

exposure. They are stored in fatty tissues from which they are slowly released into the blood stream. Laboratory studies show the material to be embryotoxic and fetotoxic.

EMERGENCY AND FIRST AID PROCEDURES

SWALLOWING: Do not induce vomiting. Seek prompt medical attention.

SKIN: Remove contaminated clothing and wipe excess material from

skin. Wash skin thoroughly with soap and water.

INHALATION: Remove to fresh air. Give artificial respiration if not

breathing. Oxygen may be given if breathing is difficult.

Call a physician.

EYES: Flush eyes thoroughly with water for a least 15 minutes. Seek

medical attention if discomfort persists.

NOTES TO PHYSICIAN: There is no specific antidote. Treatment of overexposure

should be directed to the control of symptoms and the

clinical condition.

* VI. REACTIVITY DATA

STABILITY: Stable

CONDITIONS TO AVOID: Avoid high temperatures over 550 Deg. F.

INCOMPATIBILITY

(MATERIALS TO AVOID): Oxidizing agents.

(REACTIVITY DATA continued on next page)

HAZARDOUS COMBUSTION OR

DECOMPOSITION PRODUCTS: Contact with an open flame or an electric arc can

generate carbon monoxide, carbon dioxide, and hydrogen chloride. Dioxins and furans may be produced if exposed to high temperature conditions.

HAZARDOUS POLYMERIZATION: Will not occur.

CONDITIONS TO AVOID: None

STEPS TO BE TAKEN IF MATERIAL

IS RELEASED OR SPILLED:

Nofify UNISON immediately. Collect spills with absorbent solids or contain and pump to drums for disposal. If frozen, shovel into drums.

WASTE DISPOSAL METHOD: Contains PCBs. Incinerate in facilities designed to

handle halogenated emissions gases, and approved for PCB waste which are operated in accordance with

Federal, State, and Local regulations.

* VIII. SPECIAL PROTECTION INFORMATION

RESPIRATORY PROTECTION: Full face or half face respirator with organic

canister.

VENTILATION: General (mechanical) room ventilation is expected to be

satisfactory. Use local exhaust to capture vapor, mists or fumes, if necessary. Provide greater than 60 feet per minute hood face velocity for confined spaces. Provide ventilation sufficient to prevent exceeding recommended exposure limit or buildup of explosive concentrations of vapor in air. Use explosion-proof equipment. No smoking or open lights.

PROTECTIVE GLOVES: Gloves - Approved Viton, milled nitrile or milled

neoprene

Other - Approved Saranex laminated Tyvek clothing as

appropriate.

EYE PROTECTION: Monogoggles.

OTHER PROTECTIVE EQUIPMENT: Shower, eye bath, chemical resistant apron and

boots.

PRECAUTIONS TO BE TAKEN IN HANDLING AND STORAGE:

WARNING: CAUSES EYE AND SKIN IRRITATION.

HARMFUL IF INHALED. HARMFUL IF SWALLOWED.

HARMFUL IF ABSORBED THROUGH SKIN. CAUSES CANCER IN LABORATORY ANIMALS.

Do not swallow.
Avoid breathing vapor.
Avoid contact with eyes, skin, and clothing.
Keep container closed.
Use with adequate ventilation.
Wash thoroughly after handling.

- FOR INDUSTRY USE ONLY -

OTHER PRECAUTIONS: Avoid discharge to the environment.

NOTE: While UNISON believes that the data contained herein are factual and the opinions expressed are those of qualified experts regarding the results of the tests conducted, the data are not to be taken as a warranty or representation for which UNISON assumes legal responsibility. They are offered solely for your consideration, investigation, and verification. Any use of these data and information must be determined by the user to be in accordance with applicable Federal, State and local laws and regulations.

EMERGENCY PHONE NUMBER

1-800-UCC-HELP

THIS NUMBER IS AVAILABLE DAYS, NIGHTS, WEEKENDS AND HOLIDAYS

MATERIAL SAFETY DATA SHEET

Prepared by- FS REVISION MARCH 92

Date of Preparation- 04-08-92

For:

Manufacturer: CHEMICAL SOLVENTS, INC.

Address : 3751 Jennings Road

Cleveland, Ohio 44109

Te tephanet:

(216) 741-9310

Night: (216) 741-9310

Emergancy4: (800) 424-9300(Chemtrec) National: (800) **3**62-0693

SECTION I PRODUCT IDENTIFICATION

anufacturer's Code Identification: RPTRI

Product Class:

rade Name: RECLAIMED TRICHLORDETHYLENE

HMIS Information:

Health- 3

Flammability- 1

Rwactivity- 0

AZARD INDEX: 4= Severe 3= Serious 2= Moderate 1= Slight O= Least

<u>怀</u> SECTION II HAZARDOUS INGREDIENTS

TRICHLOROETHYLENE

01

CAS# 79-01-6

XPOSURE LIMIT:

OSHA TWA (final): 50 FPM

ACGIH TLV/TWA:

50 FPM

OSHA CEILING:

200 PPM

SECTION III PHYSICAL DATA

Boiling Range: High- 188.0 F

Law-188.0 F

PPOT Pressure: 58.00 MMHC 668 F

Por Density: Heavier Than Air

Evaporation Rate: Faster than Butyl Acetate

Weight per Gallon: 12.20

Volatile by Weight:

100.00

Physical State: LIQUID

Appearance: CLEAR

or: SOLVENT

or Threshold: N/A

PH: N/A

Fineezing Foint: N/A

ter Solubility: SLIGHT

Coefficient of Water/Oil Distribution: N/A

CHEMICAL SOLVENTS, INC.

MATERIAL SAFETY DATA SHEET

RECLAIMED TRICHLOROETHYLENE

SECTION IV FIRE AND EXPLOSION DATA

Lammability Classification: N/A

DOT: N/A

EXTINGUISHING MEDIA

se CO2, dry chemical, foam, or water fog. Water may be helpful b cool containers.

UNUSUAL FIRE AND EXPLOSION HAZARD:

f this blend is flammable or combustible (flashpoint below 200),

b not weld or cut on containers.

SECTION V HEALTH HAZARD & FIRST AID

FIRST AID:

NHALATION: Move victim to fresh air. If breathing has stopped administer artificial respiration. Seek medical attention.

KIN: Remove contaminated clothing. Wash well with soap and water or at least 15 minutes. Launder contaminated clothing before wearing.

EYES: Flush eyes immediately with water for at least 15 minutes.

Lf irritation persists seek medical attention.

IGESTION: Do not induce vomiting. Seek medical attention immediately.

HEALTH TOXICITY:

E CONTACT: May cause temporary irritation with temporary corneal injury. INGESTION: Single dose toxicity is low to moderate. If vomiting occurs bis product can be aspirated into the lungs, which can cause chemical eumonia and systemic effects.

HALATION: Major potential route of exposure. Minimal effects observed below 1,000 ppm, dizziness, drowsiness, and throat irritation at levels ove 1,000 ppm. Unconsciousness and death possible at levels above 10,000 m.

SKIN CONTACT: Contact with this product may cause irritation, defatting, the skin and dermatitis. Absorption through the skin is possible contact is prolonged.

OVERALL HEALTH EFFECTS: This product has NOT been found to be a carcinogen Qr a potential carcinogen by the NTF, IARC or OSHA.

SECTION VI REACTIVITY DATA.

SMARILITY:Stable HAZARDOUS FOLYMERIZATION: Will not occur. ZARDOUS DECOMPOSITION FRODUCTS: Hydogen chloride, phosgene and chloring INCOMPATABILITIES: Strong oxidizers, barium, lithium,magnesium & titanium. Amoid aluminum in pressurized fluid systems.

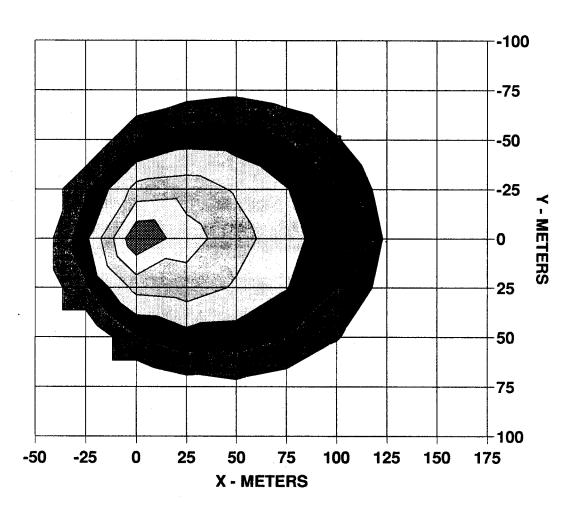
CENDITIONS TO AVOID: Hot surfaces that could cause thermal decomposition.

我们就只要你就就没有我们就没有我们就就就没有我们就没有我们的我们也也没有我们的我们就没有我们的的事情就会就是我们就没有我们的现在分 CHEMICAL SOLVENTS, INC. RPTRI MATERIAL SAFETY DATA SHEET Page 3 RECLAIMED TRICHLOROETHYLENE 我的_我我的我们就就就就就就是我们的我们的的,我们的的,我们就没有这些的的,我们就没有的的,我们就没有的的,我们就会会会就会会会会会会会会会会会会。 SECTION VII EMERGENCY RESPONSE & DISPOSAL DISPOSAL: Dispose of this product in accordance with local, state and federal regulations. Call CHEMICAL SOLVENTS, INC. for recycling or disposal. The EMERGENCY RESPONSE GUIDEBOOK number is: PRECAUTIONS IN CASE OF A SPILL OR RELEASE: Stop and contain spill if possible. Use trained personnel. Keep out of drains and waterways. Pump into drums and contact Chemical Solvents Inc. for disposal or recycling. SECTION VIII PERSONAL PROTECTION EQUIPMENT VENTILATION / RESPIRATORS: Use ventilation as required to control vapors concentrations. Avoid prolonged or repeated breathing of vapors. If exposure exceeds TLV use a NIOSH approved respirator to prevent over exposure. EYES: Wear splash goggles or a face shield when handling. SKIN: Wear solvent resistant cloves such as Viton. Use solvent resistant poots, apron, and headgear if splashing is possible. SECTION IX STORAGE & TRANSPORTATION DOT SHIPPING CLASSIFICATION: (49CFR 172.101) Trichloroethylene Solution, ORM-A UN 1710 HANDLING AND STORING PRECAUTIONS store labeled, sealed containers in a cool, dry, well ventilated area. SECTION X Section 313 Toxic Chemicals Ihis product contains the following toxic chemicals subject to the eporting requirements of section 313 of the Emergency Planning and community Right-To-Know Act of 1986 and of 40 CFR 372: hemical CAS Number Weight X TRICHLOROETHYLENE 79-01-6 90 TO 100% THE INFORMATION CONTAINED HEREIN IS INFORMATION RECEIVED FROM OUR RAW MATERIAL SUFFLIERS AND OTHER SOURCES AND IS BELIEVED TO BE RELIABLE. THIS DATA IS NOT TO BE TAKEN AS A WARRANTY OR REPRESENTATION FOR WHICH CHEMICAL SOLVENTS

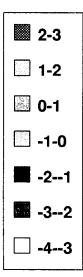
INC. ASSUMES LEGAL-RESPONSIBILITY.

APPENDIX E

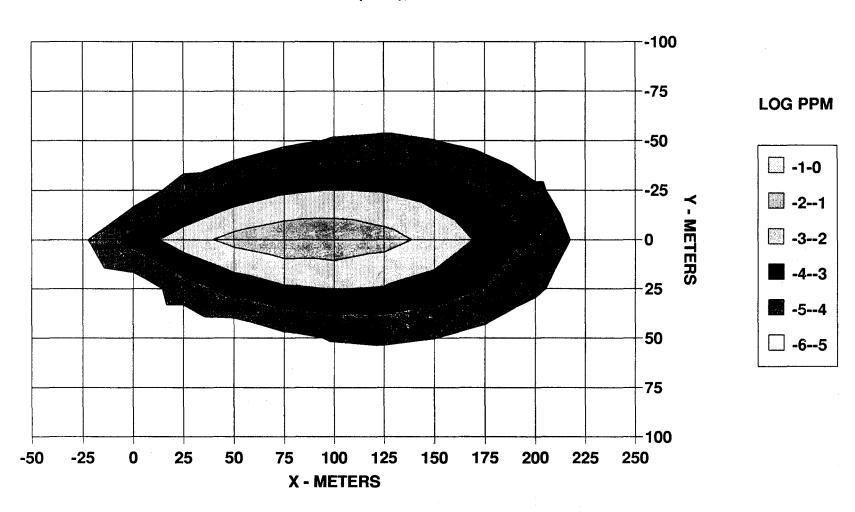
TCE PLUME PREDICTION YEAR 11 (1994), Z=0



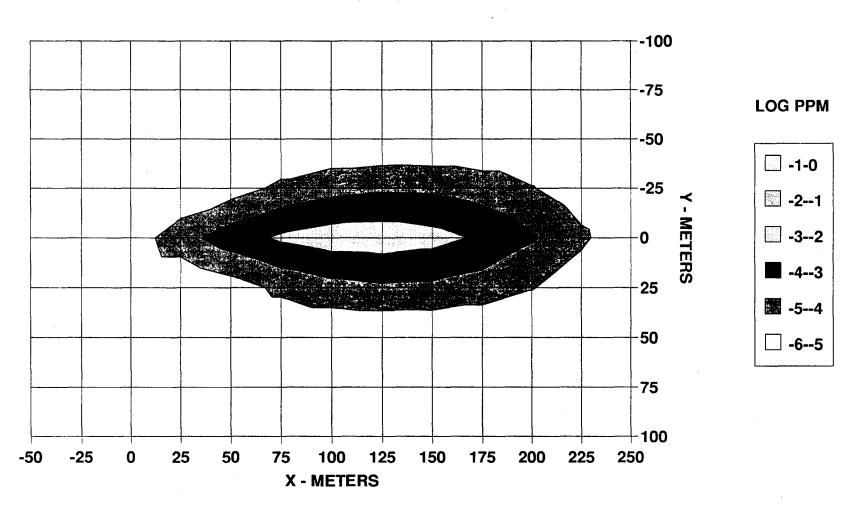
LOG PPM



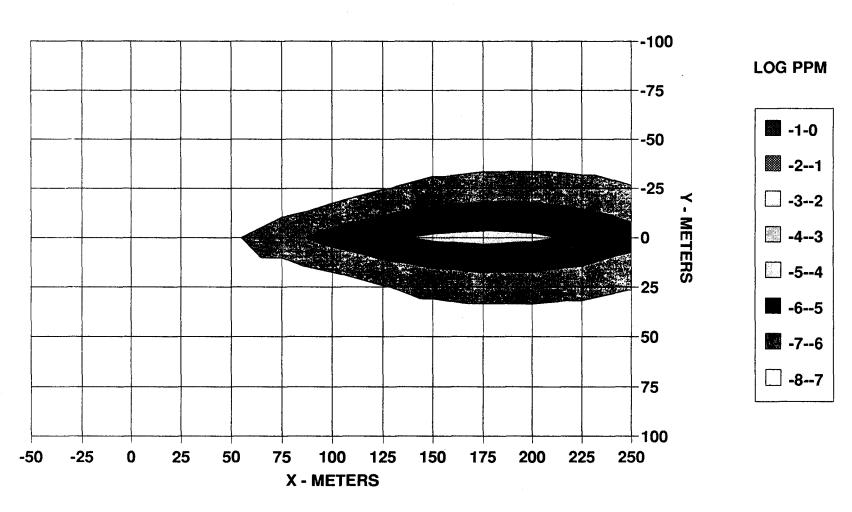
TCE PLUME PREDICTION YEAR 26 (2009), Z=0



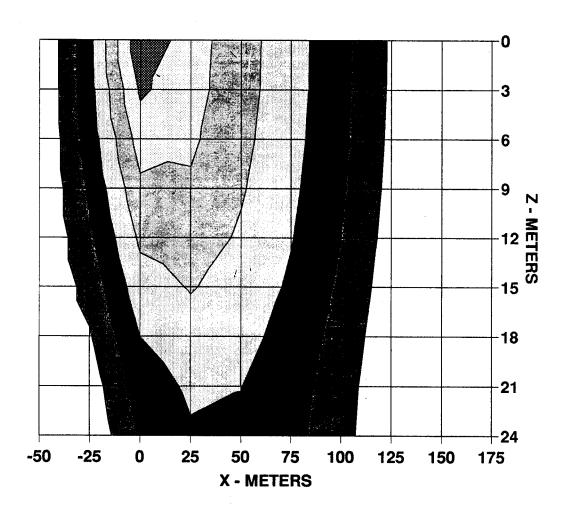
TCE PLUME PREDICTION YEAR 31 (2014), Z=0



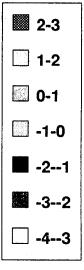
TCE PLUME PREDICTION YEAR 41 (2024), Z=0



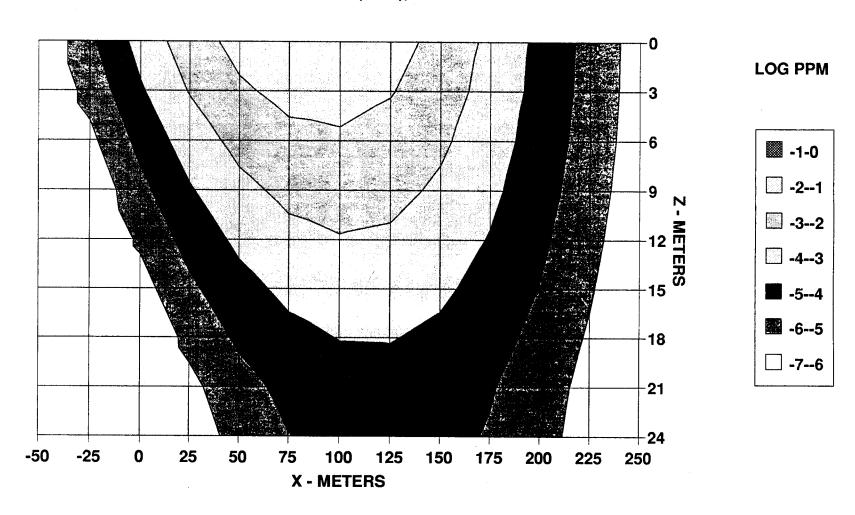
TCE PLUME PREDICTION YEAR 11 (1994), Y=0



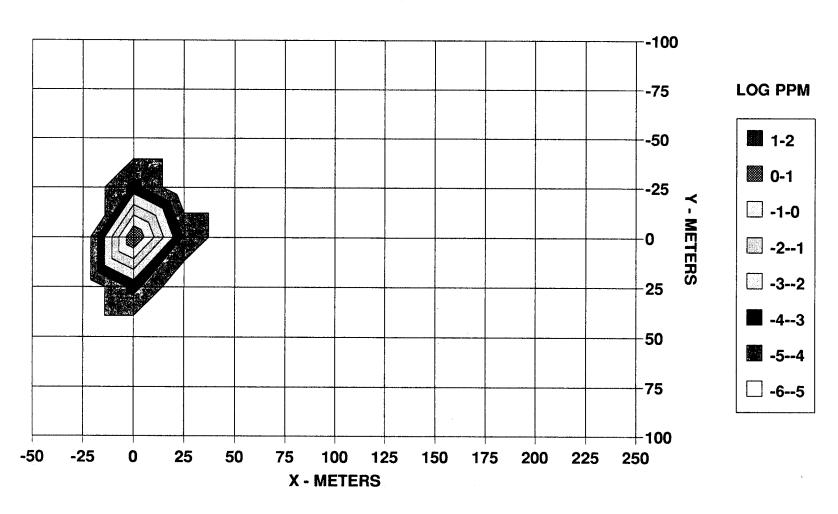
LOG PPM



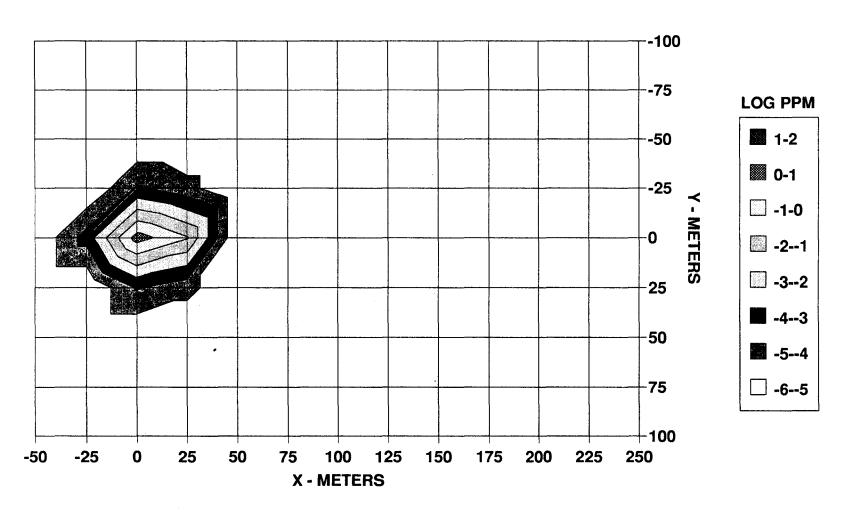
TCE PLUME PREDICTION YEAR 26 (2009), Y=0



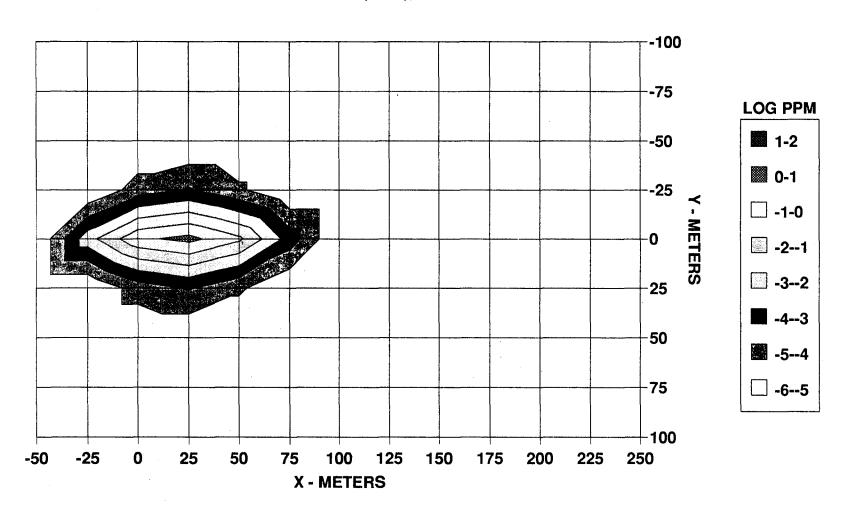
PCB PLUME PREDICTION YEAR 11 (1994), Z=0



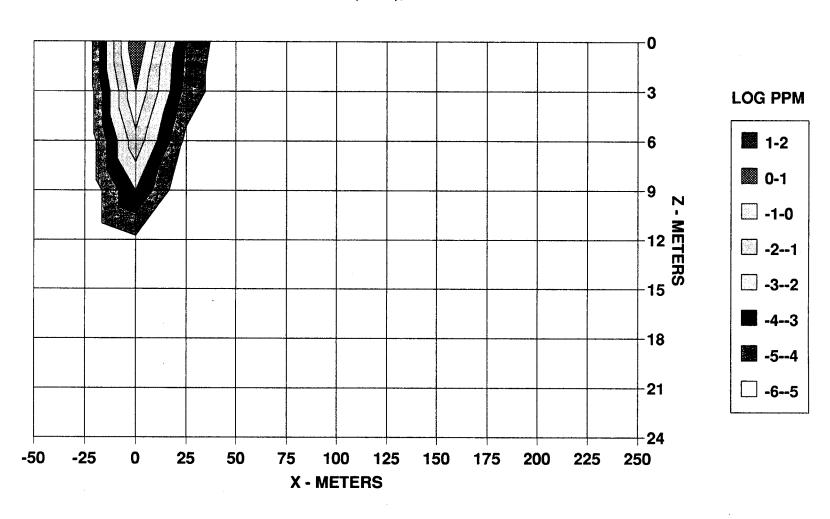
PCB PLUME PREDICTION YEAR 26 (2009), Z=0



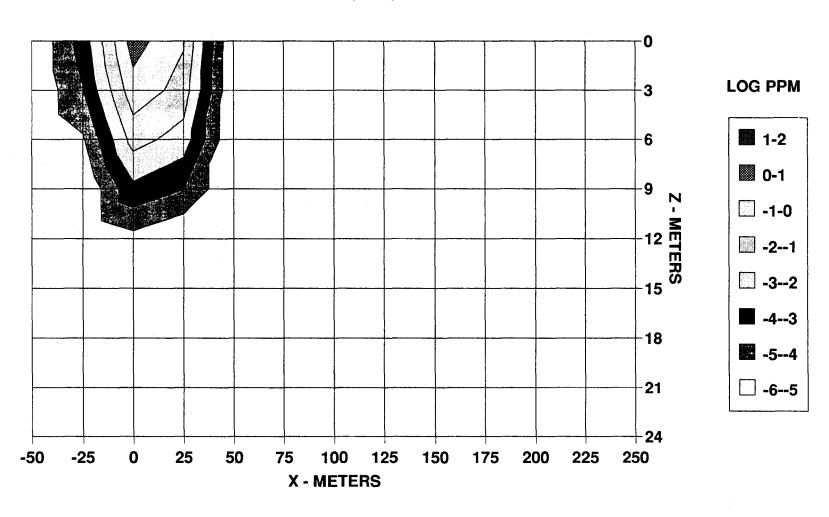
PCB PLUME PREDICTION YEAR 71 (2054), Z=0



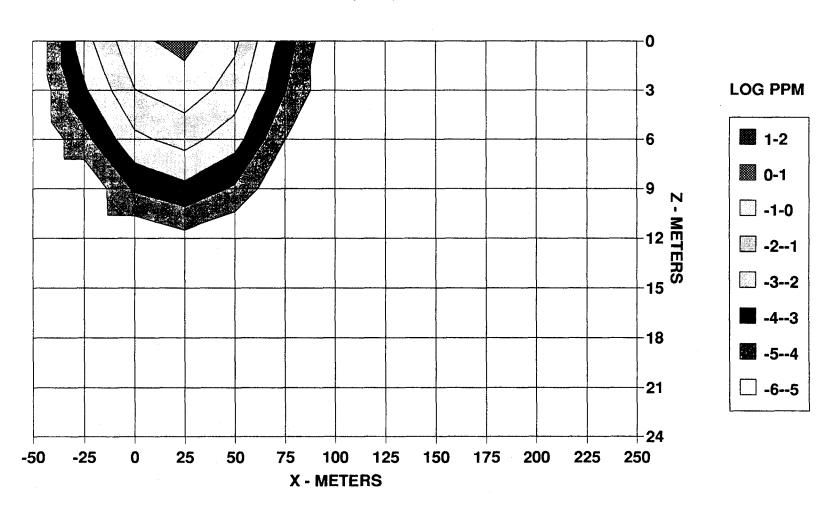
PCB PLUME PREDICTION YEAR 11 (1994), Y=0



PCB PLUME PREDICTION YEAR 26 (2009), Y=0



PCB PLUME PREDICTION YEAR 71 (2054), Y=0



APPENDIX F

Calculations Regarding the Transport of Chemicals Due to the Flood (3/94)

The objective of the calculations was to estimate the extent of subsurface contamination by TCE and PCB based on equilibrium relationships between soil and water. From the data collected, the total mass of contaminants that were contained in the source volume (i.e., the volume of excavation) were calculated. From there, an estimation was made as to the amount of contamination that could have possibly spread due to the flood event last year. These results should only be used as a first estimate of contaminant transfer and in no way implies any type of kinetic information.

Assumptions

- Contamination is uniform across the dimensions of the excavation (12' X 10' X 50'), and this volume accounts for the source of contamination.
- Organic fraction in the soil is 1.5%.
- Soil porosity = 28% of which 25 % is water filled.
- Soil bulk density = 1.4
- During flood scenario, 50% of source contamination was affected.
- The PCBs consisted of 12.5% Aroclor 1242 and 87.5% Aroclor 1260 (1:7), and the mixture has characteristics weighted in the same ratio.
- Due to the co-solvent effect of TCE in the groundwater, PCB is 100 times more soluble than it would be in a "pure" solution.

Contaminant concentrations at the source

TCE Soil conc.(measured) = 200 ppm TCE Water conc.(calculated) = 206 ppm

PCB Soil conc.(measured) = 200 ppm PCB Water conc.(calculated) = = 0.072 ppm or 72 ppb

Contaminant Masses

TCE in soil = 48 kg
TCE in water = 2.5 kg
total TCE in source volume = 50 kg
(95% of the TCE is in the soil phase.)

PCB in soil = 48 kg PCB in water = 0.85 g total PCB in volume = 48 kg (99.9+% of the PCB is in the soil phase.) During the flood scenario, about 5 kg TCE and 1g PCB could have entered the groundwater at the source and been able to migrate downwards. Even at extended solubilities, PCB has an extremely high affinity for the soil phase; thus, only TCE could have migrated to any great extent. At the 60 ppm TCE action level, this represents about 84 m³ of groundwater. High groundwater levels may have facilitated this type of transport in the past. In the soil underneath the present excavation (13' below footer) there is no obvious correlation between TCE and PCB soil concentrations.

APPENDIX G

PCB Risk Calculations

Exposure to Groundwater

Based on exposure at closest off-site access

Total Exposure

= Intake from ingestion of water

+ Intake from inhalation of volatiles from water

Exposure Assumptions:

Concentration in ground water at point of exposure (C) 0.0000111 mg/L Carcinogenic Slope Factor, inhaled (SFi) NA Carcinogenic Slope Factor, oral (SFo) 7.7 (mg/kg-day)-1 Adult Body Weight (BW) 70 kg Averaging Time (AT) 70 year Exposure Frequency (EF) 350 days/year Exposure Duration (ED) 30 years Daily in-door air inhalation rate (IRa) 15 m3/day

Daily water ingestion rate (IRw) 2 L/day Volatilization factor (K) 0.5 L/m3

Total Risk =

EF x ED x C x { (SFo x IRw) + (SFi x K x IRa) }

BW x AT x 365 days/year

TOTAL RISK FROM GROUNDWATER =

1.00E-06

Since no value for SFi exists, the (SFi x K x IRa) statement in the above equation is equal to 0 as per EPA guidance.

SENSITIVITY ANALYSIS FOR PCB IN WATER

Conc. (ppb)	Risk
0.01	9.04E-07
0.5	4.52E-05
1	9.04E-05
2	1.81E-04
5	4.52E-04
10	9.04E-04
100	9.04E-03
1000	9.04E-02